Proposed Subdivision Brush Creek Estate - Precinct 2, Stage 2 Site Classification

Kenakan Street, Edgeworth

NEW18P-0170F-AB
17 January 2022

Qualtest
LABORATORY (NSW) PTY LTD

GEOTECHNICAL I LABORATORY I EARTHWORKS I QUARRY I CONSTRUCTION MATERIAL TESTING

17 January 2022

McCloy Edgeworth Pty Ltd
Suite 2, Ground Floor, 317 Hunter Street
NEWCASTLE NSW 2300

Attention: Mr Harry Thomson

Dear Sir

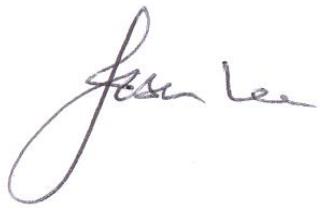
RE: PROPOSED SUBDIVISION - BRUSH CREEK ESTATE – PRECINCT 2, STAGE 2 KENAKAN STREET, EDGEWORTH SITE CLASSIFICATION (LOTS 201 TO 209)

Please find enclosed our geotechnical report for Lots 201 to 209 within Precinct 2, Stage 2 of the Brush Creek Estate residential subdivision, located at Kenakan Street, Edgeworth.

The report includes recommendations for Site Classification in accordance with AS2870-2011, "*Residential Slabs and Footings*" following the completion of site regrading earthworks.

If you have any questions regarding this report, please do not hesitate to contact Ben Bunting, Shannon Kelly or the undersigned.

For and on behalf of Qualtest Laboratory (NSW) Pty Ltd



Jason Lee
Principal Geotechnical Engineer

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1.0 Introduction

Qualtest Laboratory NSW Pty Ltd (Qualtest) is pleased to present this geotechnical report on behalf of McCloy Edgeworth Pty Ltd (McCloy), for Precinct 2, Stage 2, of the Brush Creek Estate residential subdivision, located at Kenakan Street, Edgeworth.

Based on the brief and drawings provided by the client, Stage 2 is understood to include 9 residential allotments (Lots 201 to 209), as shown on the attached sales plan provided by McCloy.

The scope of work for the geotechnical investigation included providing site classification with respect to reactive soils, in accordance with the requirements of AS2870-2011 '*Residential Slabs and Footings*', following completion of site regrade works.

This report presents the results of the field work investigations and laboratory testing, and provides recommendations for the scope outlined above.

2.0 Desktop Study

The scope of work has included a review of the following reports completed by Qualtest:

- Site Classification, 'Proposed Subdivision, Brush Creek Estate – Precinct 2, Stage 1, Bootaring Boulevard, Edgeworth, (Report Reference: NEW18P-0170D-AC, dated 14 October 2021);
- Geotechnical Assessment, 'Proposed Subdivision, Brush Creek Estate – Precinct 2, Transfield Avenue, Edgeworth, (Report Reference: NEW18P-0170A-AA.Rev1, dated 4 March 2020);
- Level 1 Site Re-grade Assessment Report, 'Brush Creek Estate – Precinct 2 Stage 2, Edgeworth', (Report Reference: NEW20P-0011F-AA, dated 17 December 2021).

This report includes a summary of selected results from the previous reports. Reference should be made to the reports outlined above for further details of site description, subsurface conditions, field work conducted, engineering logs of test pits / boreholes, laboratory testing results, site supervision and density testing carried out.

3.0 Field Work

Field work investigations were carried out on 8 December, 2021 and comprised of:

- DBYD search, review of plans, and visual check of proposed test locations for the presence of underground services;
- Site walkover to make observations of surface features at the property and in the immediate surrounding area;
- Excavation of 10 boreholes (BH201 to BH209, and BH205A), using a 2.7 tonne excavator equipped with a 300mm diameter auger attachment. Boreholes were terminated at depths of between 0.91m and 3.50m;
- Undisturbed samples (U50 tubes) were taken for subsequent laboratory testing; and,
- Boreholes were backfilled with the excavation spoil and compacted using the excavator auger and tracks.

Investigations were carried out by an experienced Geotechnical Engineer from Qualtest who located the boreholes, carried out the sampling and testing, produced field logs of the boreholes, and made observations of the site surface conditions.

Engineering logs of the boreholes are presented in Appendix A.

Approximate borehole locations are shown on the attached Figure AB1. Boreholes were located in the field relative to existing site features including topographic features, lot boundaries, existing developments and trees.

4.0 Site Description

4.1 Site Regrade Works

Following initial site visits, stripping assessments and recommendations performed on 2 June 2021, site re-grading filling works within Stage 2 residential lots was conducted between 17 June 2021 and 9 August 2021.

Re-grade works included filling within Lots 202 to 209.

Prior to filling, re-grade areas were stripped of topsoil and unsuitable material to expose the suitable natural foundation profile. Preparation works were then performed, which consisted of tyning, re-conditioning and re-compaction of the stripped surface, prior to filling with approved site fill to design finish levels.

Filling was performed using site stockpiled material won from excavations cut from around the site. The fill material could generally be described as mixtures of Residual (CI-CH) Sandy CLAY, medium to high plasticity, brown / red / grey in colour, with fine to coarse grained Sand and Gravel, along with Extremely Weathered (EW) Siltstone / Sandstone, pale yellow / brown / white in colour, blended with minor quantities of on-site pale brown Colluvium.

The approximate depth of fill placed ranged in the order of 0.3m to about 3.0m, with the deepest areas being associated with an old gully depression within Lot 205 to 207, and also adjacent to existing retaining walls along the site boundaries between Stage 1 and 2 lots.

The approximate maximum depth of fill placed over the lots excluding topsoil was in the order of:

- 1.8m – Lot 202 to 204;
- 3.0m – Lot 205 to 207;
- 1.5m – Lot 208 to 209.

The approximate extent of lot re-grade works for this stage of the development is shown on the attached Figure AB1.

The fill was compacted in maximum lifts of 0.3m thickness. Any unsuitable or deleterious material within the fill was removed by hand or mechanical means prior to final compaction of the material.

As the geotechnical testing authority engaged for the project, Qualtest state that the re-grading works performed within Stage 2 (as detailed in the site regrade report), was carried out to Level 1 criteria as defined in Clause 8.2 – Section 8, of AS3798-2007, "*Guidelines on Earthworks for Commercial and Residential Developments*". Refer to site regrade letter referenced in Section 2.0 for further details.

The recommendations of this report are based on the understanding that any existing lot re-grade works are limited to the controlled earthworks supervised by Qualtest, and placement of low reactivity topsoil material such that total depth of topsoil and uncontrolled fill does not exceed 0.4m. Qualtest should be informed without delay if additional earthworks are known to have been carried out.

At the time of the field investigations on 8 December 2021, several fill stockpiles were remaining on Lots 201 to 209. It is understood and expected that the fill stockpiles will be removed prior to development on the lots.

4.2 Surface Conditions

The site comprises Precinct 2, Stage 2 of the proposed residential subdivision known as Brush Creek Estate, located at Kenakan Street, Edgeworth, as shown on Figure AB1 attached.

The site is bounded by future Stage 4 to the west, to the south by existing Stage 1, to the north by proposed future stages of the development, and to the east by Bootaring Boulevard and in turn by undeveloped bushland.

Trafficability was judged to be good by way of 4WD vehicle along the existing sealed roads.

Photographs of the site taken on the day of the site investigations are shown below.

Photograph 1: From near western boundary of Lot 201, facing northeast.

Photograph 2: From near western boundary of Lot 201, facing east.

Photograph 3: From north-western corner of Lot 202, facing south.

Photograph 4: From north-western corner of Lot 202, facing west (along Kenakan Street).

Photograph 5: From northern boundary of Lot 206, facing east (along Kenakan Street).

Photograph 6: From northern boundary of Lot 206, facing southeast.

Photograph 7: From near south-eastern corner of Lot 207, facing northeast.

Photograph 8: From near south-eastern corner of Lot 207, facing east.

Photograph 9: From near south-eastern corner of Lot 207, facing west.

Photograph 10: From near south-eastern corner of Lot 207, facing northwest.

4.3 Subsurface Conditions

Reference to the 1:100,000 Newcastle Coalfield Regional Geology Sheet indicates the site to be underlain by the Adamstown and Boolaroo Subgroups of the Newcastle Coal Measures, which are characterised by Sandstone, Conglomerate, Siltstone, Coal, and Tuff rock types.

Table 1 presents a summary of the typical soil and rock types encountered at borehole locations during the field investigation, divided into representative geotechnical units.

Table 2 contains a summary of the distribution of the geotechnical units at the borehole locations.

TABLE 1 – SUMMARY OF GEOTECHNICAL UNITS AND SOIL / ROCK TYPES

Unit	Soil Type	Description
1A	FILL –TOPSOIL	Gravelly Sandy CLAY – medium plasticity, dark grey-brown, fine to coarse grained (mostly fine to medium grained) sand, fine to medium grained angular to sub-angular gravel.
1B	FILL – CONTROLLED	Gravelly Sandy CLAY – low to high plasticity (generally medium plasticity), pale brown with some/trace pale orange-brown and grey to white / dark brown, fine to coarse grained sand, fine to medium grained (mostly fine to medium grained) angular to sub-angular gravel. Sandy CLAY – medium to high plasticity (generally medium plasticity), grey to pale grey / brown, fine to medium grained (mostly fine grained) sand, with some fine to medium grained (mostly fine grained) angular to sub-angular gravel in places. Clayey Sandy GRAVEL – fine to medium grained, sub-rounded to sub-angular, pale brown, fines of low plasticity, fine to coarse grained sand.
2	COLLUVIUM	Not Encountered within test pits during current investigation.
3	ALLUVIUM	Not Encountered within test pits during current investigation.
4	RESIDUAL SOIL	CLAY, Sandy CLAY – medium to high plasticity, pale grey with some pale orange-brown, trace fine grained sand. Gravelly Sandy CLAY, Sandy CLAY – low to medium plasticity (generally medium plasticity), pale grey to grey and pale brown, fine to medium grained sand, fine to medium grained angular to sub-angular gravel. Gravelly Silty SAND – fine to coarse grained, pale orange-brown, fines of low plasticity, fine grained angular to sub-angular gravel.
5	EXTREMELY WEATHERED (XW) ROCK with soil properties	Silty Sandstone; breaks down into Gravelly Clayey SAND – fine to coarse grained, pale grey, fine to medium grained angular gravel, fines of low plasticity. Silty Sandstone; breaks down into Sandy CLAY – low plasticity, pale orange-brown and pale grey-to white, fine grained sand. Sandstone; breaks down into Silty SAND – fine to coarse grained, pale orange-brown, fines of low plasticity, trace fine grained angular gravel.
6	HIGHLY WEATHERED (HW) ROCK	Silty SANDSTONE – fine to medium grained, pale grey to pale brown and pale orange-brown to red-brown, estimated very low to medium strength with some high strength layers. Pebbly SANDSTONE – fine to medium grained, pale orange-brown, estimated low strength.

TABLE 2 – SUMMARY OF GEOTECHNICAL UNITS ENCOUNTERED AT BOREHOLE LOCATIONS

Location	UNIT 1A FILL-TOPSOIL	UNIT 1B FILL - CONTROLLED	UNIT 2 SLOPEWASH / COLLUVIUM	UNIT 3 ALLUVIUM	UNIT 4 RESIDUAL SOIL	UNIT 5 XW ROCK	UNIT 6 HW ROCK
	Depth (m)						
Current Investigation (December 2021)							
BH201	0.00 - 0.30	-	-	-	0.30 - 0.90 1.60 - 1.90	0.90 - 1.60 1.90 - 1.95*	
BH202	-	0.00 - 0.75	-	-	0.75 - 0.95	-	0.95 - 1.00*
BH203	-	0.00 - 1.55	-	-	-	-	1.55 - 1.90*
BH204	-	0.00 - 1.65	-	-	-	-	1.65 - 1.70*
BH205	-	0.00 - 2.80	-	-	2.80 - 3.50	-	-
BH205A	-	0.00 - 1.90	-	-	1.90 - 2.80	-	2.80 - 2.90
BH206	-	0.00 - 2.40	-	-	-	2.40 - 2.60	-
BH207	-	0.00 - 1.25	-	-	-	-	1.25 - 1.30*
BH208	-	0.00 - 0.90	-	-	-	-	0.90 - 0.91*
BH209	-	0.00 - 1.15	-	-	-	-	1.15 - 1.20*
Previous Investigation (Ref: NEW18P-0170D-AC, dated 14 October 2021)							
BH124	-	0.00 - 0.60	-	-	0.60 - 1.50	1.50 - 1.70	1.70*
BH125	0.00 - 0.30	0.30 - 0.90	-	-	0.90 - 2.00	-	-
BH126	0.00 - 0.30	0.30 - 0.60	-	-	0.60 - 2.00		-
BH127	0.00 - 0.30	0.30 - 0.80	-	-	0.80 - 2.00	-	-

Location	UNIT 1A FILL-TOPSOIL	UNIT 1B FILL - CONTROLLED	UNIT 2 SLOPEWASH / COLLUVIUM	UNIT 3 ALLUVIUM	UNIT 4 RESIDUAL SOIL	UNIT 5 XW ROCK	UNIT 6 HW ROCK
	Depth (m)						
BH128	0.00 - 0.40	0.40 - 1.50	-	-	1.50 - 1.58	-	1.58 - 1.60*
BH129	0.00 - 0.15	0.15 - 1.50	-	-	1.50 - 1.80	-	1.80 - 2.00
BH130	0.00 - 0.40	-	-	-	0.40 - 1.20	1.20 - 2.00	
BH131	0.00 - 0.20	-	-	-	0.20 - 0.60	-	0.60 - 0.62*
BH132	0.00 - 0.15	-	-	-	0.15 - 0.35		0.35 - 0.40*
BH133	0.00 - 0.15	-	-	-	0.15 - 0.35	0.35 - 0.38	0.38 - 0.40*
Previous Investigation (Ref: NEW18P-0170A-AA.Rev1, 4 March 2020) – Prior to site regrade works							
TPP09	0.00 - 0.20	-	-		0.20 - 1.20	1.20 - 2.65	-
TPP10	0.00 - 0.15	-	0.15 - 0.35		0.35 - 1.00	-	1.00 - 1.15*
TPP15	0.00 - 0.10	-	0.10 - 0.30		0.30 - 0.70	-	0.70 - 0.95*

NOTES:
* = Practical refusal or refusal of 2.7 tonne excavator with auger drill attachment met on Highly Weathered Rock.
^ = Very slow progress of 2.7 tonne excavator with auger drill attachment met on Extremely to Highly Weathered Rock.
\# = Practical refusal of hand auger met on Extremely to Highly Weathered Rock.
Preliminary investigation encountered no fill material; topsoil noted as FILL-TOPSOIL Unit 1A for simplicity.

Groundwater levels or inflows were not encountered in boreholes during the limited time that they remained open on the day of the field investigations.

It should be noted that groundwater conditions can vary due to rainfall and other influences including regional groundwater flow, temperature, permeability, recharge areas, surface condition, and subsoil drainage.

5.0 Laboratory Testing

Samples collected during the field investigations were returned to our NATA accredited Newcastle Laboratory for testing which comprised of:

- (8 no.) Shrink / Swell tests; and
- (2 no.) Atterberg Limits tests.

Two shrink/swell tests were replaced by Atterberg Limits classification tests due to the friable nature of the soils.

Results of the laboratory testing are presented in Appendix B, with a summary of the Shrink/Swell and Atterberg Limits test results presented in Table 3 and Table 4, respectively, which also include results from previous testing in the area.

TABLE 3 – SUMMARY OF SHRINK/SWELL TESTING RESULTS

Location	Depth (m)	Material Description	I_{ss} (%)
Current Investigation (October 2021)			
BH201	0.50 - 0.65	(CH) CLAY	0.5
BH202	0.60 - 0.75	FILL: (CI) Gravelly Sandy CLAY	1.8
BH203	0.40 - 0.55	FILL: (CI) Gravelly Sandy CLAY	0.5
BH203	1.40 - 1.55	FILL: (CH) Gravelly Sandy CLAY	0.9
BH204	0.80 - 1.00	FILL: (CL) Gravelly Sandy CLAY	0.3
BH205	0.70 - 0.85	FILL: (CI) Gravelly Sandy CLAY	0.9
BH207	0.60 - 0.85	FILL: (CH) Sandy CLAY	0.6
BH208	0.60 - 0.80	FILL: (CH) Sandy CLAY	0.5
Previous Investigation (Ref: NEW18P-0170D-AC, dated 14 October 2021)			
BH125	0.50 - 0.70	FILL: (CI) Gravelly Sandy CLAY	0.6
BH126	0.40 - 0.50	FILL: (CI) Sandy CLAY	2.4
BH129	0.40 - 0.60	FILL: (CI) Gravelly Sandy CLAY	0.4
BH130	0.70 - 0.90	(CH) Sandy CLAY	1.1
BH131	0.40 - 0.60	FILL: (CI) Gravelly Sandy CLAY	0.8
BH133	0.20 - 0.35	(CI) Gravelly Sandy CLAY	3.7
Previous Investigation (Ref: NEW18P-0170A-AA.Rev1, 4 March 2020)			
TTP09	0.40 - 0.50	(CH) CLAY	5.1

TABLE 4 – SUMMARY OF ATTERBERG LIMITS TESTING RESULTS

Location	Depth (m)	Material Description	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	Linear Shrinkage (%)
Current Investigation (December 2021)						
BH206	0.50 - 0.65	FILL: (CL) Gravelly Sandy CLAY	31	16	15	8.0
BH209	0.60 - 0.75	FILL: (CI) Gravelly Sandy CLAY	46	17	29	-
Previous Investigation (Ref: NEW18P-0170D-AC, dated 14 October 2021)						
BH124	0.80 - 0.95	(CH) CLAY	50	17	33	12.0
BH127	0.30 - 0.45	FILL: (CI) Gravelly Sandy CLAY	40	19	21	9.0
BH128	0.80 - 1.00	FILL: (CI) Gravelly Sandy CLAY	37	16	21	8.0
Previous Investigation (Ref: NEW18P-0170A-AA.Rev1, 4 March 2020)						
TPP15	0.30 - 0.50	(CI) Gravelly Sandy CLAY	44	20	24	9.0

The results of the Shrink/Swell and Atterberg Limits laboratory testing indicate that the residual soils tested from the site generally contain fines of medium and medium to high plasticity, and that the fill materials generally contain fines of low to high plasticity.

6.0 Site Classification to AS2870-2011

Based on the results of the field work, laboratory testing and site regrade works conducted, residential lots located within Precinct 2, Stage 2 of the Brush Creek Estate residential subdivision, as shown on the attached Figure AB1, are classified in their current condition in accordance with AS2870-2011 *'Residential Slabs and Footings'*, as shown in Table 5.

TABLE 5 – SITE CLASSIFICATION TO AS2870-2011

Lot Numbers	Site Classification
201 to 204 and 207 to 209	**H1**
205 to 206	**H2**

Notes:	Localised fill stockpiles remained on Lots 201 to 209 at the time of the field investigations, the approximate locations of which are shown on Figure AB1. Site classifications provided herein are made on the understanding that the fill stockpiles will be removed prior to sales / development of the lots, such that remaining topsoil and/or uncontrolled fill depths on lots is less than 0.4m.

	If any localised areas of topsoil and/or uncontrolled fill of depths greater than 0.4m are encountered during construction, footings should be designed in accordance with engineering principles for **Class 'P'** sites.

A characteristic free surface movement of 40mm to 60mm is estimated for the lots classified as **Class 'H1'** in their existing condition.

A characteristic free surface movement of 60mm to 75mm is estimated for the lots classified as **Class 'H2'** in their existing condition.

The effects of changes to the soil profile by additional cutting and filling and the effects of past and future trees should be considered in selection of the design value for differential movement.

If site re-grading works involving cutting or filling are performed after the date of this assessment, the classification may change and further advice should be sought.

Footings for the proposed development should be designed and constructed in accordance with the requirements of AS2870-2011.

The classification presented above assumes that:

- All footings are founded in controlled fill (if applicable) or in the residual clayey soils or rock below all non-controlled fill, topsoil material and root zones, and fill under slab panels meets the requirements of AS2870-2011, in particular, the root zone must be removed prior to the placement of fill materials beneath slabs;
- The performance expectations set out in Appendix B of AS2870-2011 are acceptable, and that site foundation maintenance is undertaken to avoid extremes of wetting and drying;
- Footings are to be founded outside of or below all zones of influence resulting from existing or future service trenches;
- The constructional and architectural requirements for reactive clay sites set out in AS2870-2011 are followed;
- Adherence to the detailing requirement outlined in Section 5 of AS2870-2011 '*Residential Slabs and Footings'* is essential, in particular Section 5.6, '*Additional requirements for Classes M, H1, H2 and E sites'* including architectural restrictions, plumbing and drainage requirements; and,
- Site maintenance complies with the provisions of CSIRO Sheet BTF 18, "*Foundation Maintenance and Footing Performance: A Homeowner's Guide*", a copy of which is attached in Appendix C.

All structural elements on all lots should be supported on footings founded beneath all uncontrolled fill, layers of inadequate bearing capacity, soft/loose, wet or other potentially deleterious material.

If any localised areas of uncontrolled fill of depths greater than 0.4m are encountered during construction, footings should be designed in accordance with engineering principles for Class 'P' sites.

7.0 Limitations

The findings presented in the report and used as the basis for recommendations presented herein were obtained using normal, industry accepted geotechnical design practices and standards. To our knowledge, they represent a reasonable interpretation of the general conditions of the site.

The extent of testing associated with this assessment is limited to discrete test locations. It should be noted that subsurface conditions between and away from the test locations may be different to those observed during the field work and used as the basis of the recommendations contained in this report.

If subsurface conditions encountered during construction differ from those given in this report, further advice should be sought without delay.

Data and opinions contained within the report may not be used in other contexts or for any other purposes without prior review and agreement by Qualtest. If this report is reproduced, it must be in full.

If you have any further questions regarding this report, please do not hesitate to contact Shannon Kelly, Ben Bunting, or the undersigned.

For and on behalf of Qualtest Laboratory (NSW) Pty Ltd.

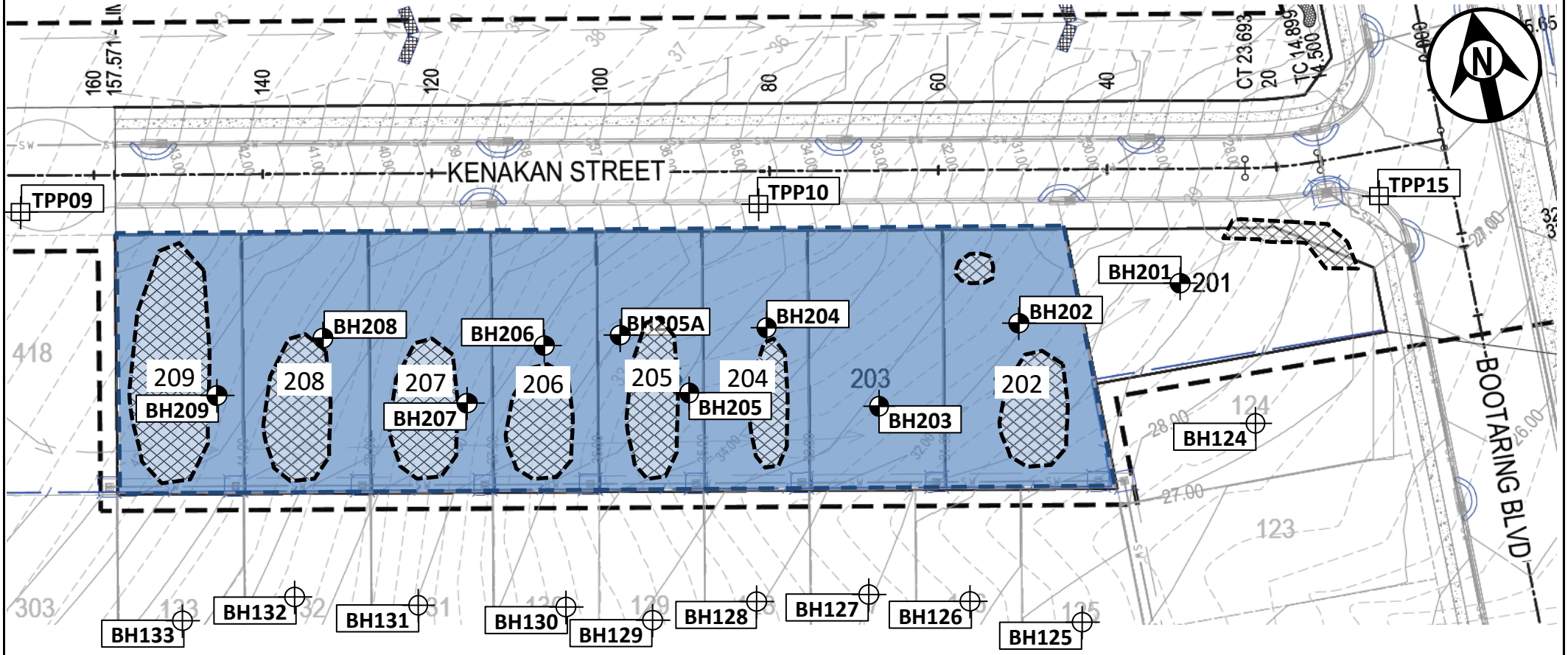
Jason Lee
Principal Geotechnical Engineer

FIGURE AB1:

Site Plan and Approximate Test Locations

SALE PLAN:

Sales Plan for Stage 2 Brush Creek



LEGEND:

- Approximate borehole test location (Current investigation)
- / Approximate previous borehole / test pit location (Qualtest, 2020 to 2021)
- Approximate location and extent of regrade area (Fill)
- Approximate extent and location of fill stockpiles observed during field work.

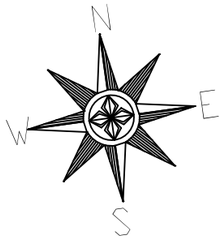
Based on Site Plan prepared by ACOR Consultants Pty Ltd (Project No. NSW201400, Dwg No. C211.03, Iss. B, dated 10/06/2021)

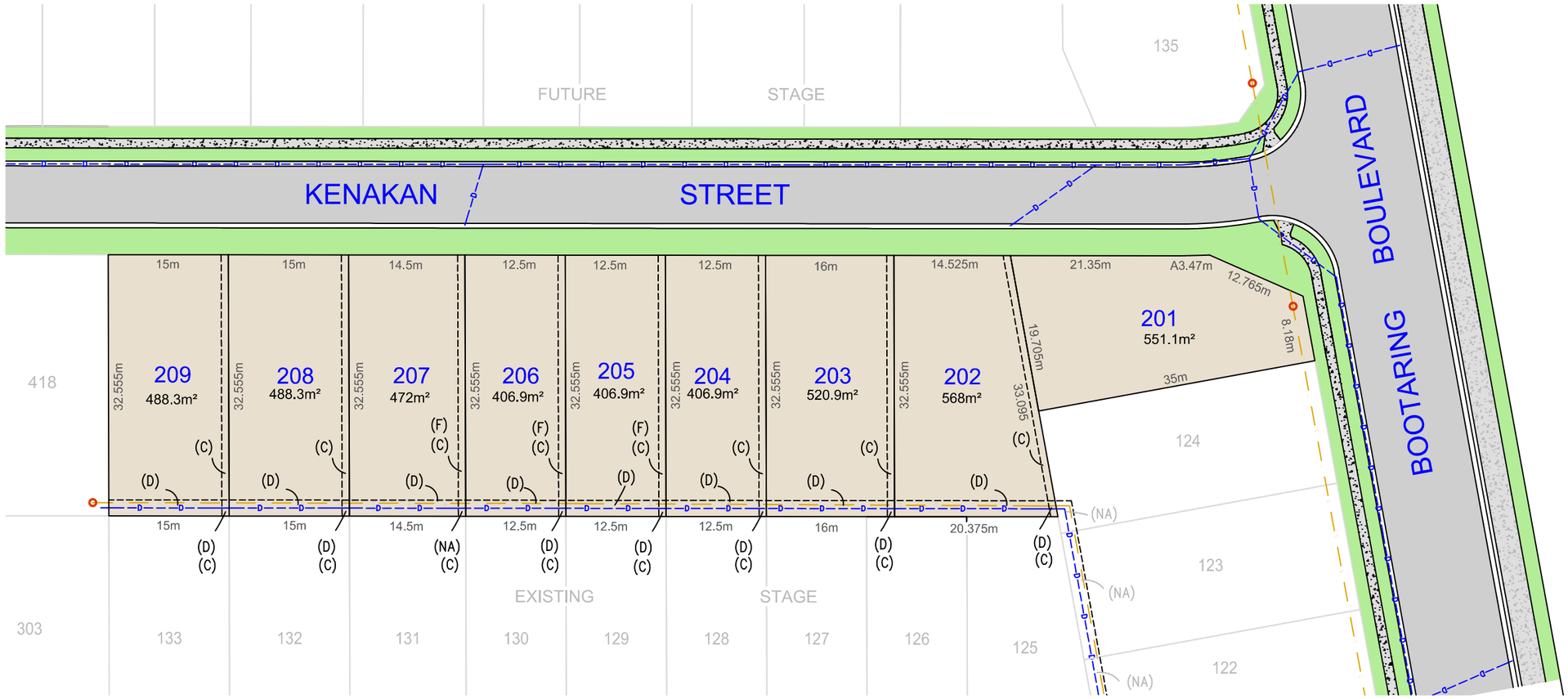
Qualtest LABORATORY (NSW) PTY LTD

Client:	MCCLOY EDGEWORTH PTY LTD	Drawing No:	FIGURE AB1
Project:	BRUSH CREEK ESTATE - PRECINCT 2, STAGE 2	Project No:	NEW18P-0170F
Location:	KENAKAN STREET, EDGEWORTH	Scale:	N.T.S.
Title:	SITE PLAN AND APPROXIMATE TEST LOCATIONS	Date:	17/01/2022

SALES PLAN FOR STAGE 2 BRUSH CREEK

BRUSH CREEK
— LAKE MACQUARIE —





(D) EASEMENT TO DRAIN WATER 2 WIDE

(C) RESTRICTION ON THE USE OF LAND 0.9 WIDE

(F) EASEMENT FOR CONSTRUCTION AND MAINTENANCE 0.9 WIDE

(NA) EASEMENT TO DRAIN WATER 2 WIDE

LEGEND	
— —D— —	PROPOSED STORMWATER DRAINAGE LINE
— S —	PROPOSED SEWER LINE
◎	PROPOSED SEWER MANHOLE

LOCALITY PLAN

McCLOYGROUP

APPENDIX A:

Results of Field Investigations

ENGINEERING LOG - BOREHOLE

BOREHOLE NO: BH201

CLIENT: MCCLOY EDGEWORTH PTY LTD
PAGE: 1 OF 1
PROJECT: BRUSH CREEK - PRECINCT 2, STAGE 2
JOB NO: NEW18P-0170F
LOCATION: KENAKAN STREET, EDGEWORTH
LOGGED BY: BB
DATE: 8/12/21

DRILL TYPE: 2.7 TONNE EXCAVATOR
SURFACE RL:
BOREHOLE DIAMETER: 300 mm
DATUM:

Drilling and Sampling					Material description and profile information					Field Test		
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics,colour,minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Structure and additional observations
AD/T	Not Encountered	0.50m U50 0.65m		0.5		CI	FILL-TOPSOIL: Gravelly Sandy CLAY - medium plasticity, dark grey-brown, fine to coarse grained (mostly fine to medium grained) sand, fine to medium grained angular to sub-angular gravel.					FILL - TOPSOIL
							0.30m CLAY - medium to high plasticity, pale grey with some pale orange-brown, trace fine grained sand.	M ~ w_P	VSt	HP	320	RESIDUAL SOIL
						CH				HP	300	
				1.0		SC	0.90m Extremely Weathered Silty Sandstone with soil properties; breaks down into Gravelly Clayey SAND - fine to coarse grained, pale grey, fine to medium grained angular gravel, fines of low plasticity.		VD			EXTREMELY WEATHERED ROCK
							1.00m Silty SANDSTONE - fine to medium grained, pale brown to pale grey, estimated very low to low strength.	D				HIGHLY WEATHERED ROCK
				1.5		CL	1.40m Extremely Weathered Silty Sandstone with soil properties; breaks down into Sandy CLAY - low plasticity, pale orange-brown and pale grey to white, fine grained sand.					EXTREMELY WEATHERED ROCK
							1.60m CLAY - medium plasticity, pale grey and pale orange-brown with some red-brown, trace fine grained sand.	M < w_P	H / Fb			RESIDUAL SOIL
						CI	1.90m					
							1.95m Silty SANDSTONE - fine grained, pale orange-brown to red-brown and pale grey, estimated medium strength.	D				HIGHLY WEATHERED ROCK
				2.0			Hole Terminated at 1.95 m Practical Refusal					

LEGEND:

Water
- Water Level (Date and time shown)
- Water Inflow
- Water Outflow

Strata Changes
- Gradational or transitional strata
- Definitive or distict strata change

Notes, Samples and Tests

U_{50}	50mm Diameter tube sample
CBR	Bulk sample for CBR testing
E	Environmental sample (Glass jar, sealed and chilled on site)
ASS	Acid Sulfate Soil Sample (Plastic bag, air expelled, chilled)
B	Bulk Sample

Field Tests

PID	Photoionisation detector reading (ppm)
DCP(x-y)	Dynamic penetrometer test (test depth interval shown)
HP	Hand Penetrometer test (UCS kPa)

Consistency

		UCS (kPa)
VS	Very Soft	<25
S	Soft	25 - 50
F	Firm	50 - 100
St	Stiff	100 - 200
VSt	Very Stiff	200 - 400
H	Hard	>400
Fb	Friable	

Density

V	Very Loose	Density Index <15%
L	Loose	Density Index 15 - 35%
MD	Medium Dense	Density Index 35 - 65%
D	Dense	Density Index 65 - 85%
VD	Very Dense	Density Index 85 - 100%

Moisture Condition

D	Dry
M	Moist
W	Wet
W_p	Plastic Limit
W_L	Liquid Limit

ENGINEERING LOG - BOREHOLE

CLIENT: MCCLOY EDGEWORTH PTY LTD
PROJECT: BRUSH CREEK - PRECINCT 2, STAGE 2
LOCATION: KENAKAN STREET, EDGEWORTH

BOREHOLE NO: BH202
PAGE: 1 OF 1
JOB NO: NEW18P-0170F
LOGGED BY: BB
DATE: 8/12/21

DRILL TYPE: 2.7 TONNE EXCAVATOR
BOREHOLE DIAMETER: 300 mm
SURFACE RL:
DATUM:

Drilling and Sampling					Material description and profile information					Field Test		
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics,colour,minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Structure and additional observations
AD/T	Not Encountered	0.60m U50 0.75m		0.5		CI	FILL: Gravelly Sandy CLAY - medium plasticity, pale brown with some pale orange-brown, fine to coarse grained sand, fine to medium grained angular to sub-angular gravel.	M ~ w_P	VSt	HP	230	FILL - CONTROLLED
										HP	250	
							0.75m Sandy CLAY - medium plasticity, pale grey to grey, fine to medium grained sand.	M < w_P		HP	380	RESIDUAL SOIL
				1.0		CI	0.95m 1.00m Silty SANDSTONE - fine to medium grained, grey trace pale orange-brown, estimated medium strength.	D				HIGHLY WEATHERED ROCK
							Hole Terminated at 1.00 m Practical Refusal					
				1.5								
				2.0								

LEGEND:

Water
- Water Level (Date and time shown)
- Water Inflow
- Water Outflow

Strata Changes
- Gradational or transitional strata
- Definitive or distict strata change

Notes, Samples and Tests

U_{50}	50mm Diameter tube sample
CBR	Bulk sample for CBR testing
E	Environmental sample (Glass jar, sealed and chilled on site)
ASS	Acid Sulfate Soil Sample (Plastic bag, air expelled, chilled)
B	Bulk Sample

Field Tests

PID	Photoionisation detector reading (ppm)
DCP(x-y)	Dynamic penetrometer test (test depth interval shown)
HP	Hand Penetrometer test (UCS kPa)

Consistency

		UCS (kPa)
VS	Very Soft	<25
S	Soft	25 - 50
F	Firm	50 - 100
St	Stiff	100 - 200
VSt	Very Stiff	200 - 400
H	Hard	>400
Fb	Friable	

Density

V	Very Loose	Density Index <15%
L	Loose	Density Index 15 - 35%
MD	Medium Dense	Density Index 35 - 65%
D	Dense	Density Index 65 - 85%
VD	Very Dense	Density Index 85 - 100%

Moisture Condition

D	Dry
M	Moist
W	Wet
W_p	Plastic Limit
W_L	Liquid Limit

ENGINEERING LOG - BOREHOLE

BOREHOLE NO: BH203

CLIENT: MCCLOY EDGEWORTH PTY LTD
PAGE: 1 OF 1
PROJECT: BRUSH CREEK - PRECINCT 2, STAGE 2
JOB NO: NEW18P-0170F
LOCATION: KENAKAN STREET, EDGEWORTH
LOGGED BY: BB
DATE: 8/12/21

DRILL TYPE: 2.7 TONNE EXCAVATOR
SURFACE RL:
BOREHOLE DIAMETER: 300 mm
DATUM:

Drilling and Sampling					Material description and profile information					Field Test		
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics,colour,minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Structure and additional observations
AD/T	Not Encountered	0.40m U50 0.55m 1.40m U50 1.55m		0.5 1.0 1.5		CI CI CH	FILL: Gravelly Sandy CLAY - medium plasticity, grey-brown trace pale orange-brown and grey to white, fine to coarse grained (mostly fine to medium grained) sand, fine to medium grained (mostly fine grained) angular to sub-angular gravel. 0.80m FILL: Sandy CLAY - medium plasticity, grey to pale grey, fine to medium grained (mostly fine grained) sand. 1.10m FILL: Gravelly Sandy CLAY - medium to high plasticity, dark brown, fine to coarse grained (mostly fine grained) sand. 1.55m Pebbly SANDSTONE - fine to medium grained, pale orange-brown, estimated low strength. 1.90m	M > w_P M ~ w_P M > w_P D	VSt	HP HP HP HP HP	280 380 300 350 210 250	FILL - CONTROLLED HIGHLY WEATHERED ROCK
				2.0			Hole Terminated at 1.90 m Practical Refusal					

LEGEND:

Water
- Water Level (Date and time shown)
- Water Inflow
- Water Outflow

Strata Changes
- Gradational or transitional strata
- Definitive or distict strata change

Notes, Samples and Tests

U_{50}	50mm Diameter tube sample
CBR	Bulk sample for CBR testing
E	Environmental sample (Glass jar, sealed and chilled on site)
ASS	Acid Sulfate Soil Sample (Plastic bag, air expelled, chilled)
B	Bulk Sample

Field Tests

PID	Photoionisation detector reading (ppm)
DCP(x-y)	Dynamic penetrometer test (test depth interval shown)
HP	Hand Penetrometer test (UCS kPa)

Consistency

		UCS (kPa)
VS	Very Soft	<25
S	Soft	25 - 50
F	Firm	50 - 100
St	Stiff	100 - 200
VSt	Very Stiff	200 - 400
H	Hard	>400
Fb	Friable	

Density

V	Very Loose	Density Index <15%
L	Loose	Density Index 15 - 35%
MD	Medium Dense	Density Index 35 - 65%
D	Dense	Density Index 65 - 85%
VD	Very Dense	Density Index 85 - 100%

Moisture Condition

D	Dry
M	Moist
W	Wet
W_p	Plastic Limit
W_L	Liquid Limit

ENGINEERING LOG - BOREHOLE

BOREHOLE NO: **BH204**
PAGE: 1 OF 1
JOB NO: NEW18P-0170F
LOGGED BY: BB
DATE: 8/12/21

CLIENT: MCCLOY EDGEWORTH PTY LTD
PROJECT: BRUSH CREEK - PRECINCT 2, STAGE 2
LOCATION: KENAKAN STREET, EDGEWORTH

DRILL TYPE: 2.7 TONNE EXCAVATOR
BOREHOLE DIAMETER: 300 mm
SURFACE RL:
DATUM:

Drilling and Sampling					Material description and profile information					Field Test		
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics,colour,minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Structure and additional observations
AD/T	Not Encountered	0.80m U50 1.00m		0.5 1.0 1.5 2.0		CL CH	FILL: Gravelly Sandy CLAY - low to medium plasticity, brown to pale brown, fine to coarse grained sand, fine to coarse grained (mostly fine to medium grained) angular to sub-angular gravel. Pale brown. 1.30m FILL: Sandy CLAY - medium to high plasticity, grey and grey-brown, fine to coarse grained sand, trace fine grained angular gravel. 1.65m 1.70m Silty SANDSTONE - fine grained, pale grey to white, estimated medium to high strength. Hole Terminated at 1.70 m Practical Refusal	M < w_P M > w_P D	H / Fb VSt	HP HP HP HP	480 520 230 250	FILL - CONTROLLED HIGHLY TO MODERATELY WEATHERED ROCK

LEGEND:

Water
- Water Level (Date and time shown)
- Water Inflow
- Water Outflow

Strata Changes
- Gradational or transitional strata
- Definitive or distict strata change

Notes, Samples and Tests

U_{50}	50mm Diameter tube sample
CBR	Bulk sample for CBR testing
E	Environmental sample (Glass jar, sealed and chilled on site)
ASS	Acid Sulfate Soil Sample (Plastic bag, air expelled, chilled)
B	Bulk Sample

Field Tests

PID	Photoionisation detector reading (ppm)
DCP(x-y)	Dynamic penetrometer test (test depth interval shown)
HP	Hand Penetrometer test (UCS kPa)

Consistency

		UCS (kPa)
VS	Very Soft	<25
S	Soft	25 - 50
F	Firm	50 - 100
St	Stiff	100 - 200
VSt	Very Stiff	200 - 400
H	Hard	>400
Fb	Friable	

Density

V	Very Loose	Density Index <15%
L	Loose	Density Index 15 - 35%
MD	Medium Dense	Density Index 35 - 65%
D	Dense	Density Index 65 - 85%
VD	Very Dense	Density Index 85 - 100%

Moisture Condition

D	Dry
M	Moist
W	Wet
W_p	Plastic Limit
W_L	Liquid Limit

ENGINEERING LOG - BOREHOLE

BOREHOLE NO: BH205
PAGE: 1 OF 2
JOB NO: NEW18P-0170F
LOGGED BY: BB
DATE: 8/12/21

CLIENT: MCCLOY EDGEWORTH PTY LTD
PROJECT: BRUSH CREEK - PRECINCT 2, STAGE 2
LOCATION: KENAKAN STREET, EDGEWORTH

DRILL TYPE: 2.7 TONNE EXCAVATOR
BOREHOLE DIAMETER: 300 mm
SURFACE RL:
DATUM:

Drilling and Sampling					Material description and profile information					Field Test		
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics,colour,minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Structure and additional observations
AD/T	Not Encountered	0.70m U50 0.85m		0.5 1.0 1.5 2.0		CI CH CI	FILL: Gravelly Sandy CLAY - medium plasticity, pale brown, fine to coarse grained sand, fine to medium grained angular to sub-angular gravel. Dark brown. 1.20m FILL: Sandy CLAY - medium to high plasticity, pale brown, fine to coarse grained sand, with some fine to medium grained (mostly fine grained) angular gravel. 1.70m FILL: Sandy CLAY - medium plasticity, dark brown, fine to coarse grained sand, with some fine grained angular to sub-angular gravel.	M > w_P M < w_P M > w_P	VSt	HP HP HP HP HP HP	280 290 270 230 250 310 210	FILL - CONTROLLED

LEGEND:

Water
- Water Level (Date and time shown)
- Water Inflow
- Water Outflow

Strata Changes
- Gradational or transitional strata
- Definitive or distict strata change

Notes, Samples and Tests

U_{50}	50mm Diameter tube sample
CBR	Bulk sample for CBR testing
E	Environmental sample (Glass jar, sealed and chilled on site)
ASS	Acid Sulfate Soil Sample (Plastic bag, air expelled, chilled)
B	Bulk Sample

Field Tests

PID	Photoionisation detector reading (ppm)
DCP(x-y)	Dynamic penetrometer test (test depth interval shown)
HP	Hand Penetrometer test (UCS kPa)

Consistency

		UCS (kPa)
VS	Very Soft	<25
S	Soft	25 - 50
F	Firm	50 - 100
St	Stiff	100 - 200
VSt	Very Stiff	200 - 400
H	Hard	>400
Fb	Friable	

Density

V	Very Loose	Density Index <15%
L	Loose	Density Index 15 - 35%
MD	Medium Dense	Density Index 35 - 65%
D	Dense	Density Index 65 - 85%
VD	Very Dense	Density Index 85 - 100%

Moisture Condition

D	Dry
M	Moist
W	Wet
W_p	Plastic Limit
W_L	Liquid Limit

ENGINEERING LOG - BOREHOLE

CLIENT: MCCLOY EDGEWORTH PTY LTD
PROJECT: BRUSH CREEK - PRECINCT 2, STAGE 2
LOCATION: KENAKAN STREET, EDGEWORTH

BOREHOLE NO: BH205
PAGE: 2 OF 2
JOB NO: NEW18P-0170F
LOGGED BY: BB
DATE: 8/12/21

DRILL TYPE: 2.7 TONNE EXCAVATOR
BOREHOLE DIAMETER: 300 mm
SURFACE RL:
DATUM:

Drilling and Sampling					Material description and profile information					Field Test		
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics,colour,minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Structure and additional observations
AD/T	Not Encountered					CI	FILL: Sandy CLAY - medium plasticity, dark brown, fine to coarse grained sand, with some fine grained angular to sub-angular gravel. *(continued)*	M > w_P	VSt	HP	200	
				3.0		SM	2.80m Gravelly Silty SAND - fine to coarse grained, pale orange-brown, fines of low plasticity, fine grained angular to sub-angular gravel.	D	VD			RESIDUAL SOIL / POSSIBLE EXTREMELY WEATHERED ROCK / POSSIBLE FILL
						CI	3.30m Sandy CLAY - medium plasticity, red-brown with some grey and white, fine to medium grained sand.	M ~ w_P	VSt	HP	370	RESIDUAL SOIL / POSSIBLE FILL
				3.5		CL	3.40m Sandy CLAY - low to medium plasticity, pale brown to pale grey-brown, fine to coarse grained (mostly fine grained) sand, with some fine grained angular to sub-angular gravel.					
							3.50m Hole Terminated at 3.50 m Limit Of Reach					
				4.0								
				4.5								

LEGEND:

Water
- Water Level (Date and time shown)
- Water Inflow
- Water Outflow

Strata Changes
- Gradational or transitional strata
- Definitive or distict strata change

Notes, Samples and Tests

U_{50}	50mm Diameter tube sample
CBR	Bulk sample for CBR testing
E	Environmental sample (Glass jar, sealed and chilled on site)
ASS	Acid Sulfate Soil Sample (Plastic bag, air expelled, chilled)
B	Bulk Sample

Field Tests

PID	Photoionisation detector reading (ppm)
DCP(x-y)	Dynamic penetrometer test (test depth interval shown)
HP	Hand Penetrometer test (UCS kPa)

Consistency

		UCS (kPa)
VS	Very Soft	<25
S	Soft	25 - 50
F	Firm	50 - 100
St	Stiff	100 - 200
VSt	Very Stiff	200 - 400
H	Hard	>400
Fb	Friable	

Density

V	Very Loose	Density Index <15%
L	Loose	Density Index 15 - 35%
MD	Medium Dense	Density Index 35 - 65%
D	Dense	Density Index 65 - 85%
VD	Very Dense	Density Index 85 - 100%

Moisture Condition

D	Dry
M	Moist
W	Wet
W_p	Plastic Limit
W_L	Liquid Limit

ENGINEERING LOG - BOREHOLE

BOREHOLE NO: BH205A

CLIENT: MCCLOY EDGEWORTH PTY LTD
PAGE: 1 OF 2
PROJECT: BRUSH CREEK - PRECINCT 2, STAGE 2
JOB NO: NEW18P-0170F
LOCATION: KENAKAN STREET, EDGEWORTH
LOGGED BY: BB
DATE: 8/12/21

DRILL TYPE: 2.7 TONNE EXCAVATOR
SURFACE RL:
BOREHOLE DIAMETER: 300 mm
DATUM:

Drilling and Sampling					Material description and profile information					Field Test		
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics,colour,minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Structure and additional observations
AD/T	Not Encountered			0.0			FILL: Gravelly Sandy CLAY - medium plasticity, dark grey-brown, fine to coarse grained sand, fine to medium grained angular gravel.	M > w_P	VSt	HP	300	FILL - CONTROLLED
							Pale brown to pale orange-brown.					
				0.5		CI				HP	280	
							Dark grey-brown.	M ~ w_P				
				1.0			1.00m			HP	260	
						CI	FILL: Sandy CLAY - medium plasticity, pale brown, fine to coarse grained sand, with some fine grained sub-rounded to angular gravel.					
							1.30m	M ~ w_P				
				1.5		CL	FILL: Gravelly Sandy CLAY - low to medium plasticity, pale brown, fine to coarse grained sand, fine grained sub-rounded to angular gravel.			HP	290	
							1.90m					RESIDUAL SOIL / EXTREMELY WEATHERED ROCK / POSSIBLE FILL
				2.0		CL	Gravelly Sandy CLAY - low to medium plasticity, pale grey-brown, fine to coarse grained (mostly fine grained) sand, fine to medium grained angular to sub-angular gravel.	M < w_P	H / Fb			
							2.35m					
						CH	Sandy CLAY - medium to high plasticity, grey to dark grey trace brown, fine to coarse grained sand, trace fine grained rounded to sub-angular gravel.	M ~ w_P	VSt	HP	240	RESIDUAL SOIL / POSSIBLE FILL

LEGEND:

Water
- Water Level (Date and time shown)
- Water Inflow
- Water Outflow

Strata Changes
- Gradational or transitional strata
- Definitive or distict strata change

Notes, Samples and Tests

U_{50}	50mm Diameter tube sample
CBR	Bulk sample for CBR testing
E	Environmental sample (Glass jar, sealed and chilled on site)
ASS	Acid Sulfate Soil Sample (Plastic bag, air expelled, chilled)
B	Bulk Sample

Field Tests

PID	Photoionisation detector reading (ppm)
DCP(x-y)	Dynamic penetrometer test (test depth interval shown)
HP	Hand Penetrometer test (UCS kPa)

Consistency

		UCS (kPa)
VS	Very Soft	<25
S	Soft	25 - 50
F	Firm	50 - 100
St	Stiff	100 - 200
VSt	Very Stiff	200 - 400
H	Hard	>400
Fb	Friable	

Density

V	Very Loose	Density Index <15%
L	Loose	Density Index 15 - 35%
MD	Medium Dense	Density Index 35 - 65%
D	Dense	Density Index 65 - 85%
VD	Very Dense	Density Index 85 - 100%

Moisture Condition

D	Dry
M	Moist
W	Wet
W_p	Plastic Limit
W_L	Liquid Limit

ENGINEERING LOG - BOREHOLE

BOREHOLE NO: BH205A
PAGE: 2 OF 2
JOB NO: NEW18P-0170F
LOGGED BY: BB
DATE: 8/12/21

CLIENT: MCCLOY EDGEWORTH PTY LTD
PROJECT: BRUSH CREEK - PRECINCT 2, STAGE 2
LOCATION: KENAKAN STREET, EDGEWORTH

DRILL TYPE: 2.7 TONNE EXCAVATOR
BOREHOLE DIAMETER: 300 mm
SURFACE RL:
DATUM:

Drilling and Sampling					Material description and profile information					Field Test		
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics,colour,minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Structure and additional observations
AD/T	Not Encountered					CH	Sandy CLAY - medium to high plasticity, grey to dark grey trace brown, fine to coarse grained sand, trace fine grained rounded to sub-angular gravel. *(continued)*	M ~ w_P	VSt			RESIDUAL SOIL / POSSIBLE FILL
						CH	2.70m CLAY - medium to high plasticity, grey with some red-brown, trace fine grained sand.					RESIDUAL SOIL
							2.80m Pebbly SANDSTONE - fine to coarse grained, pale orange-brown to pale brown, estimated very low to low strength.	D		HP	380	HIGHLY WEATHERED ROCK
							2.90m Estimated medium to high strength.					
				3.0			Hole Terminated at 2.90 m					
				3.5								
				4.0								
				4.5								

LEGEND:

Water
- Water Level (Date and time shown)
- Water Inflow
- Water Outflow

Strata Changes
- Gradational or transitional strata
- Definitive or distict strata change

Notes, Samples and Tests

U_{50}	50mm Diameter tube sample
CBR	Bulk sample for CBR testing
E	Environmental sample (Glass jar, sealed and chilled on site)
ASS	Acid Sulfate Soil Sample (Plastic bag, air expelled, chilled)
B	Bulk Sample

Field Tests

PID	Photoionisation detector reading (ppm)
DCP(x-y)	Dynamic penetrometer test (test depth interval shown)
HP	Hand Penetrometer test (UCS kPa)

Consistency

		UCS (kPa)
VS	Very Soft	<25
S	Soft	25 - 50
F	Firm	50 - 100
St	Stiff	100 - 200
VSt	Very Stiff	200 - 400
H	Hard	>400
Fb	Friable	

Density

V	Very Loose	Density Index <15%
L	Loose	Density Index 15 - 35%
MD	Medium Dense	Density Index 35 - 65%
D	Dense	Density Index 65 - 85%
VD	Very Dense	Density Index 85 - 100%

Moisture Condition

D	Dry
M	Moist
W	Wet
W_p	Plastic Limit
W_L	Liquid Limit

ENGINEERING LOG - BOREHOLE

BOREHOLE NO: BH206
PAGE: 1 OF 2
JOB NO: NEW18P-0170F
LOGGED BY: BB
DATE: 8/12/21

CLIENT: MCCLOY EDGEWORTH PTY LTD
PROJECT: BRUSH CREEK - PRECINCT 2, STAGE 2
LOCATION: KENAKAN STREET, EDGEWORTH

DRILL TYPE: 2.7 TONNE EXCAVATOR
BOREHOLE DIAMETER: 300 mm
SURFACE RL:
DATUM:

Drilling and Sampling					Material description and profile information					Field Test		
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics,colour,minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Structure and additional observations
AD/T	Not Encountered	0.50m U50 0.65m		0.5		CL	FILL: Gravelly Sandy CLAY - low to medium plasticity, pale brown, fine to coarse grained sand, fine to coarse grained (mostly fine to medium grained) angular gravel. Pale grey-brown. Trace cobbles.	M < w_P	H / Fb	HP HP	550 490	FILL - CONTROLLED
				1.0								
				1.5		GC	1.30m FILL: Clayey Sandy GRAVEL - fine to medium grained, sub-rounded to sub-angular, pale brown, fines of low plasticity, fine to coarse grained sand.	M	D			
						CI	1.75m FILL: Sandy CLAY - medium plasticity, pale orange-brown with some pale grey, fine to medium grained (mostly fine grained) sand.		VSt	HP	250	
				2.0		CI	2.00m FILL: Gravelly Sandy CLAY - medium plasticity, pale brown to brown, fine to coarse grained sand, fine to medium grained sub-rounded to sub-angular gravel.	M ~ w_P	VSt / Fb			
						SM	2.40m	D	VD			EXTREMELY WEATHERED ROCK

LEGEND:

Water
- Water Level (Date and time shown)
- Water Inflow
- Water Outflow

Strata Changes
- Gradational or transitional strata
- Definitive or distict strata change

Notes, Samples and Tests

U_{50}	50mm Diameter tube sample
CBR	Bulk sample for CBR testing
E	Environmental sample (Glass jar, sealed and chilled on site)
ASS	Acid Sulfate Soil Sample (Plastic bag, air expelled, chilled)
B	Bulk Sample

Field Tests

PID	Photoionisation detector reading (ppm)
DCP(x-y)	Dynamic penetrometer test (test depth interval shown)
HP	Hand Penetrometer test (UCS kPa)

Consistency

		UCS (kPa)
VS	Very Soft	<25
S	Soft	25 - 50
F	Firm	50 - 100
St	Stiff	100 - 200
VSt	Very Stiff	200 - 400
H	Hard	>400
Fb	Friable	

Density

V	Very Loose	Density Index <15%
L	Loose	Density Index 15 - 35%
MD	Medium Dense	Density Index 35 - 65%
D	Dense	Density Index 65 - 85%
VD	Very Dense	Density Index 85 - 100%

Moisture Condition

D	Dry
M	Moist
W	Wet
W_p	Plastic Limit
W_L	Liquid Limit

ENGINEERING LOG - BOREHOLE

BOREHOLE NO: **BH206**
PAGE: 2 OF 2
JOB NO: NEW18P-0170F
LOGGED BY: BB
DATE: 8/12/21

CLIENT: MCCLOY EDGEWORTH PTY LTD
PROJECT: BRUSH CREEK - PRECINCT 2, STAGE 2
LOCATION: KENAKAN STREET, EDGEWORTH

DRILL TYPE: 2.7 TONNE EXCAVATOR
BOREHOLE DIAMETER: 300 mm
SURFACE RL:
DATUM:

Drilling and Sampling					Material description and profile information					Field Test		
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics,colour,minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Structure and additional observations
AD/T						SM	2.60m Extremely Weathered Sandstone with soil properties; breaks down into Silty SAND - fine to coarse grained, pale orange-brown, fines of low plasticity, trace fine grained angular gravel. *(continued)*	D	VD			EXTREMELY WEATHERED ROCK
							Hole Terminated at 2.60 m					

3.0
3.5
4.0
4.5

LEGEND:

Water
- Water Level (Date and time shown)
- Water Inflow
- Water Outflow

Strata Changes
- Gradational or transitional strata
- Definitive or distict strata change

Notes, Samples and Tests

U_{50}	50mm Diameter tube sample
CBR	Bulk sample for CBR testing
E	Environmental sample (Glass jar, sealed and chilled on site)
ASS	Acid Sulfate Soil Sample (Plastic bag, air expelled, chilled)
B	Bulk Sample

Field Tests

PID	Photoionisation detector reading (ppm)
DCP(x-y)	Dynamic penetrometer test (test depth interval shown)
HP	Hand Penetrometer test (UCS kPa)

Consistency		UCS (kPa)
VS	Very Soft	<25
S	Soft	25 - 50
F	Firm	50 - 100
St	Stiff	100 - 200
VSt	Very Stiff	200 - 400
H	Hard	>400
Fb	Friable	

Moisture Condition	
D	Dry
M	Moist
W	Wet
W_p	Plastic Limit
W_L	Liquid Limit

Density		
V	Very Loose	Density Index <15%
L	Loose	Density Index 15 - 35%
MD	Medium Dense	Density Index 35 - 65%
D	Dense	Density Index 65 - 85%
VD	Very Dense	Density Index 85 - 100%

ENGINEERING LOG - BOREHOLE

CLIENT: MCCLOY EDGEWORTH PTY LTD
PROJECT: BRUSH CREEK - PRECINCT 2, STAGE 2
LOCATION: KENAKAN STREET, EDGEWORTH

BOREHOLE NO: BH207
PAGE: 1 OF 1
JOB NO: NEW18P-0170F
LOGGED BY: BB
DATE: 8/12/21

DRILL TYPE: 2.7 TONNE EXCAVATOR
BOREHOLE DIAMETER: 300 mm
SURFACE RL:
DATUM:

Drilling and Sampling					Material description and profile information					Field Test		
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics,colour,minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Structure and additional observations
AD/T	Not Encountered	0.60m U50 0.85m		0.5		CI	FILL: Gravelly Sandy CLAY - medium plasticity, dark grey-brown, fine to coarse grained sand, fine to coarse grained angular gravel.	M < w_P	H	HP	550	FILL - CONTROLLED
							0.40m FILL: Sandy CLAY - medium to high plasticity, pale brown and pale grey to white, fine to coarse grained sand. With some fine to medium grained angular gravel.	M > w_P	VSt	HP	350	
				1.0		CI	0.90m FILL: Gravelly Sandy CLAY - medium plasticity, pale brown, fine to coarse grained sand, fine to medium grained rounded to sub-angular gravel.	M ~ w_P	VSt - H	HP HP	450 410	
							1.25m 1.30m Silty SANDSTONE - fine grained, pale orange-brown, estimated medium to high strength.	D				HIGHLY WEATHERED ROCK
				1.5			Hole Terminated at 1.30 m Practical Refusal					
				2.0								

LEGEND:

Water
- Water Level (Date and time shown)
- Water Inflow
- Water Outflow

Strata Changes
- Gradational or transitional strata
- Definitive or distict strata change

Notes, Samples and Tests

U_{50}	50mm Diameter tube sample
CBR	Bulk sample for CBR testing
E	Environmental sample (Glass jar, sealed and chilled on site)
ASS	Acid Sulfate Soil Sample (Plastic bag, air expelled, chilled)
B	Bulk Sample

Field Tests

PID	Photoionisation detector reading (ppm)
DCP(x-y)	Dynamic penetrometer test (test depth interval shown)
HP	Hand Penetrometer test (UCS kPa)

Consistency

		UCS (kPa)
VS	Very Soft	<25
S	Soft	25 - 50
F	Firm	50 - 100
St	Stiff	100 - 200
VSt	Very Stiff	200 - 400
H	Hard	>400
Fb	Friable	

Density

V	Very Loose	Density Index <15%
L	Loose	Density Index 15 - 35%
MD	Medium Dense	Density Index 35 - 65%
D	Dense	Density Index 65 - 85%
VD	Very Dense	Density Index 85 - 100%

Moisture Condition

D	Dry
M	Moist
W	Wet
W_p	Plastic Limit
W_L	Liquid Limit

ENGINEERING LOG - BOREHOLE

BOREHOLE NO: BH208
PAGE: 1 OF 1
JOB NO: NEW18P-0170F
LOGGED BY: BB
DATE: 8/12/21

CLIENT: MCCLOY EDGEWORTH PTY LTD
PROJECT: BRUSH CREEK - PRECINCT 2, STAGE 2
LOCATION: KENAKAN STREET, EDGEWORTH

DRILL TYPE: 2.7 TONNE EXCAVATOR
BOREHOLE DIAMETER: 300 mm
SURFACE RL:
DATUM:

Drilling and Sampling					Material description and profile information					Field Test		
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics,colour,minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Structure and additional observations
AD/T	Not Encountered					CL	FILL: Gravelly Sandy CLAY - low to medium plasticity, dark grey-brown, fine to coarse grained sand, fine to coarse (mostly fine to medium grained) angular gravel.	M < w_P	H / Fb			FILL - CONTROLLED
							0.30m					
				0.5			FILL: Sandy CLAY - medium to high plasticity, pale grey to pale brown, fine to coarse grained sand, with some fine to medium grained angular gravel.			HP	330	
		0.60m				CH	Grey-brown.	M ~ w_P	VSt	HP	380	
		U50										
		0.80m								HP	350	
							0.90m					
							0.91m	D				
				1.0			Silty SANDSTONE - fine grained, pale orange-brown, estimated medium to high strength.					HIGHLY WEATHERED ROCK
							Hole Terminated at 0.91 m					
							Practical Refusal					
				1.5								
				2.0								

LEGEND:

Water
- Water Level (Date and time shown)
- Water Inflow
- Water Outflow

Strata Changes
- Gradational or transitional strata
- Definitive or distict strata change

Notes, Samples and Tests

U_{50}	50mm Diameter tube sample
CBR	Bulk sample for CBR testing
E	Environmental sample (Glass jar, sealed and chilled on site)
ASS	Acid Sulfate Soil Sample (Plastic bag, air expelled, chilled)
B	Bulk Sample

Field Tests

PID	Photoionisation detector reading (ppm)
DCP(x-y)	Dynamic penetrometer test (test depth interval shown)
HP	Hand Penetrometer test (UCS kPa)

Consistency

		UCS (kPa)
VS	Very Soft	<25
S	Soft	25 - 50
F	Firm	50 - 100
St	Stiff	100 - 200
VSt	Very Stiff	200 - 400
H	Hard	>400
Fb	Friable	

Density

V	Very Loose	Density Index <15%
L	Loose	Density Index 15 - 35%
MD	Medium Dense	Density Index 35 - 65%
D	Dense	Density Index 65 - 85%
VD	Very Dense	Density Index 85 - 100%

Moisture Condition

D	Dry
M	Moist
W	Wet
W_p	Plastic Limit
W_L	Liquid Limit

ENGINEERING LOG - BOREHOLE

CLIENT: MCCLOY EDGEWORTH PTY LTD
PROJECT: BRUSH CREEK - PRECINCT 2, STAGE 2
LOCATION: KENAKAN STREET, EDGEWORTH

BOREHOLE NO: BH209
PAGE: 1 OF 1
JOB NO: NEW18P-0170F
LOGGED BY: BB
DATE: 8/12/21

DRILL TYPE: 2.7 TONNE EXCAVATOR
BOREHOLE DIAMETER: 300 mm
SURFACE RL:
DATUM:

Drilling and Sampling					Material description and profile information					Field Test		
METHOD	WATER	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION: Soil type, plasticity/particle characteristics,colour,minor components	MOISTURE CONDITION	CONSISTENCY DENSITY	Test Type	Result	Structure and additional observations
AD/T	Not Encountered	0.60m U50 0.75m		0.5		CL	FILL: Gravelly Sandy CLAY - low to medium plasticity, dark brown, fine to coarse grained sand, fine to coarse grained angular gravel.	M < w_P	H / Fb	HP	>600	FILL - CONTROLLED
							0.40m					
				1.0		CI	FILL: Gravelly Sandy CLAY - medium plasticity, pale brown, fine to coarse grained sand, fine grained angular to sub-angular gravel.	M > w_P	VSt	HP	210	
								M ~ w_P	H / Fb			
							1.15m					
							1.20m Silty SANDSTONE - fine grained, red-brown, estimated medium strength.	D				HIGHLY WEATHERED ROCK
				1.5			Hole Terminated at 1.20 m Practical Refusal					
				2.0								

LEGEND:

Water
- Water Level (Date and time shown)
- Water Inflow
- Water Outflow

Strata Changes
- Gradational or transitional strata
- Definitive or distict strata change

Notes, Samples and Tests

U_{50}	50mm Diameter tube sample
CBR	Bulk sample for CBR testing
E	Environmental sample (Glass jar, sealed and chilled on site)
ASS	Acid Sulfate Soil Sample (Plastic bag, air expelled, chilled)
B	Bulk Sample

Field Tests

PID	Photoionisation detector reading (ppm)
DCP(x-y)	Dynamic penetrometer test (test depth interval shown)
HP	Hand Penetrometer test (UCS kPa)

Consistency

		UCS (kPa)
VS	Very Soft	<25
S	Soft	25 - 50
F	Firm	50 - 100
St	Stiff	100 - 200
VSt	Very Stiff	200 - 400
H	Hard	>400
Fb	Friable	

Density

V	Very Loose	Density Index <15%
L	Loose	Density Index 15 - 35%
MD	Medium Dense	Density Index 35 - 65%
D	Dense	Density Index 65 - 85%
VD	Very Dense	Density Index 85 - 100%

Moisture Condition

D	Dry
M	Moist
W	Wet
W_p	Plastic Limit
W_L	Liquid Limit

APPENDIX B:

Results of Laboratory Testing

QUALTEST Laboratory (NSW) Pty Ltd (20708)
2 Murray Dwyer Circuit, Mayfield West, NSW 2304
T: 02 4968 4468
F: 02 4960 9775
E: admin@qualtest.com.au
W: www.qualtest.com.au
ABN: 98 153 268 896

Report No: SSI:NEW21W-5267-S01
Issue No: 1

Shrink Swell Index Report

Client: McCloy Project Management Pty Ltd
PO Box 2214
Dangar NSW 2309

Project No.: NEW18P-0170F
Project Name: Brush Creek Subdivision - Precinct 2, Stage 2
Project Location: Bootaring Boulevard, Edgeworth

Accredited for compliance with ISO/IEC 17025-Testing.
The results of the tests, calibrations and/or measurements included in this document are traceable to Australian/national standards.
Results provided relate only to the items tested or sampled.

Approved Signatory: Brent Cullen
(Senior Geotechnician)
NATA Accredited Laboratory Number: 18686
Date of Issue: 21/12/2021

Sample Details

Sample ID: NEW21W-5267-S01
Sampling Method: The results outlined below apply to the sample as received
Material: Clay
Source: On-Site Insitu
Specification: No Specification
Sample Location: BH201 - (0.50 - 0.65m)
Date Tested: 14/12/2021
Date Sampled: 8/12/2021
Date Submitted: 10/12/2021

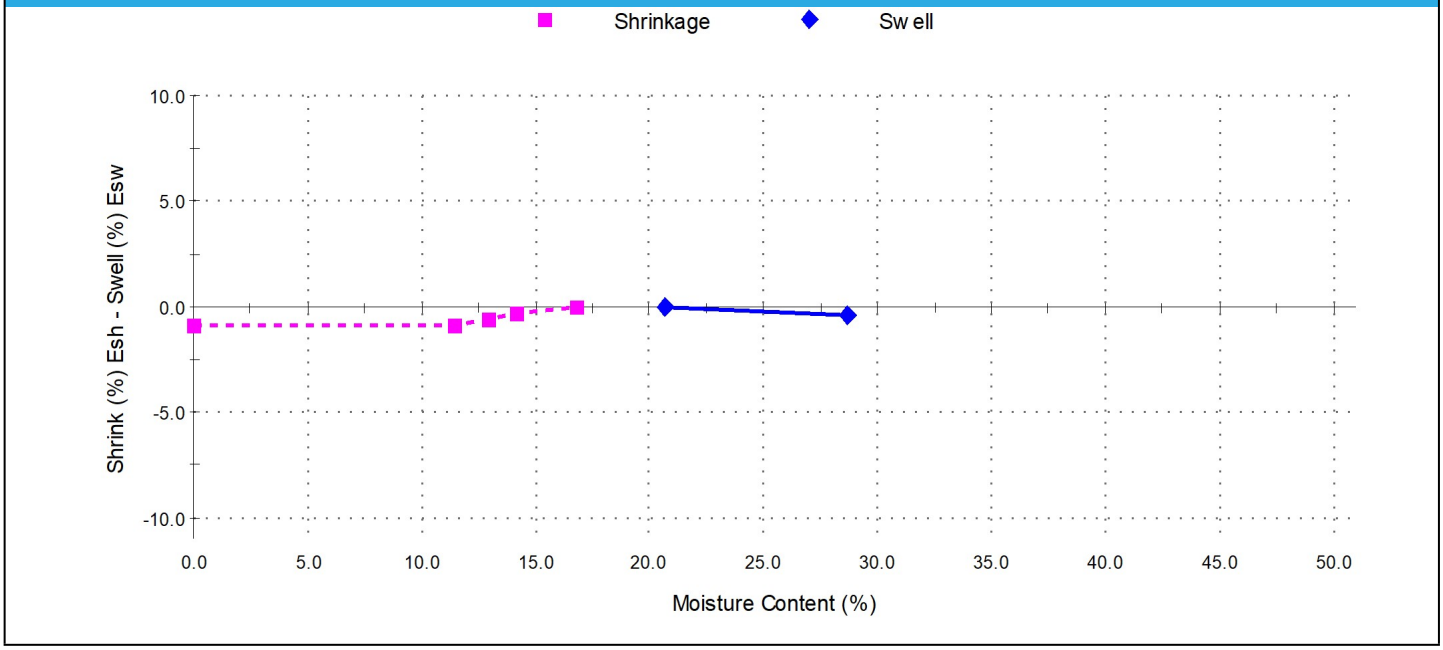
Swell Test AS 1289.7.1.1

Swell on Saturation (%):	-0.4
Moisture Content before (%):	20.7
Moisture Content after (%):	28.7
Est. Unc. Comp. Strength before (kPa):	370
Est. Unc. Comp. Strength after (kPa):	130

Shrink Test AS 1289.7.1.1

Shrink on drying (%):	0.9
Shrinkage Moisture Content (%):	16.8
Est. inert material (%):	3%
Crumbling during shrinkage:	Nil
Cracking during shrinkage:	Moderate

Shrink Swell



Shrink Swell Index - Iss (%): 0.5

Comments

QUALTEST Laboratory (NSW) Pty Ltd (20708)
2 Murray Dwyer Circuit, Mayfield West, NSW 2304
T: 02 4968 4468
F: 02 4960 9775
E: admin@qualtest.com.au
W: www.qualtest.com.au
ABN: 98 153 268 896

Report No: SSI:NEW21W-5267-S02
Issue No: 1

Shrink Swell Index Report

Client: McCloy Project Management Pty Ltd
PO Box 2214
Dangar NSW 2309

Project No.: NEW18P-0170F
Project Name: Brush Creek Subdivision - Precinct 2, Stage 2
Project Location: Bootaring Boulevard, Edgeworth

Accredited for compliance with ISO/IEC 17025-Testing.
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Approved Signatory: Brent Cullen
(Senior Geotechnician)
NATA Accredited Laboratory Number: 18686
Date of Issue: 21/12/2021

Sample Details

Sample ID: NEW21W-5267-S02
Sampling Method: The results outlined below apply to the sample as received
Material: Gravelly Sandy Clay
Source: On-Site Insitu
Specification: No Specification
Sample Location: BH202 - (0.60 - 0.75m)
Date Tested: 14/12/2021
Date Sampled: 8/12/2021
Date Submitted: 10/12/2021

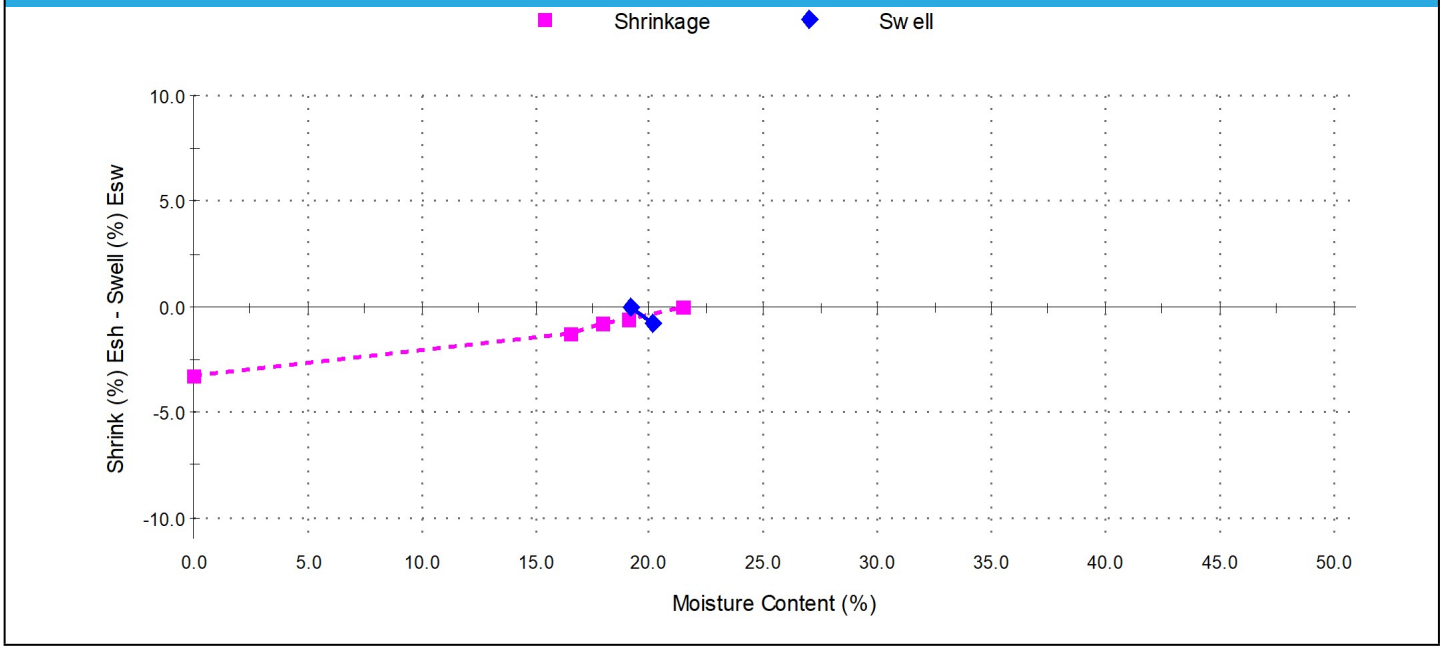
Swell Test AS 1289.7.1.1

Swell on Saturation (%):	-0.8
Moisture Content before (%):	19.2
Moisture Content after (%):	20.1
Est. Unc. Comp. Strength before (kPa):	570
Est. Unc. Comp. Strength after (kPa):	500

Shrink Test AS 1289.7.1.1

Shrink on drying (%):	3.3
Shrinkage Moisture Content (%):	21.4
Est. inert material (%):	1%
Crumbling during shrinkage:	Nil
Cracking during shrinkage:	Minor

Shrink Swell



Shrink Swell Index - Iss (%): 1.8

Comments

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2 Murray Dwyer Circuit, Mayfield West, NSW 2304
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ABN: 98 153 268 896

Report No: SSI:NEW21W-5267-S03
Issue No: 1

Shrink Swell Index Report

Client: McCloy Project Management Pty Ltd
PO Box 2214
Dangar NSW 2309

Project No.: NEW18P-0170F
Project Name: Brush Creek Subdivision - Precinct 2, Stage 2
Project Location: Bootaring Boulevard, Edgeworth

Accredited for compliance with ISO/IEC 17025-Testing.
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Results provided relate only to the items tested or sampled.

Approved Signatory: Brent Cullen
(Senior Geotechnician)
NATA Accredited Laboratory Number: 18686
Date of Issue: 21/12/2021

Sample Details

Sample ID: NEW21W-5267-S03
Sampling Method: The results outlined below apply to the sample as received
Material: Gravelly Sandy Clay
Source: On-Site Insitu
Specification: No Specification
Sample Location: BH203 - (0.40 - 0.55m)
Date Tested: 14/12/2021
Date Sampled: 8/12/2021
Date Submitted: 10/12/2021

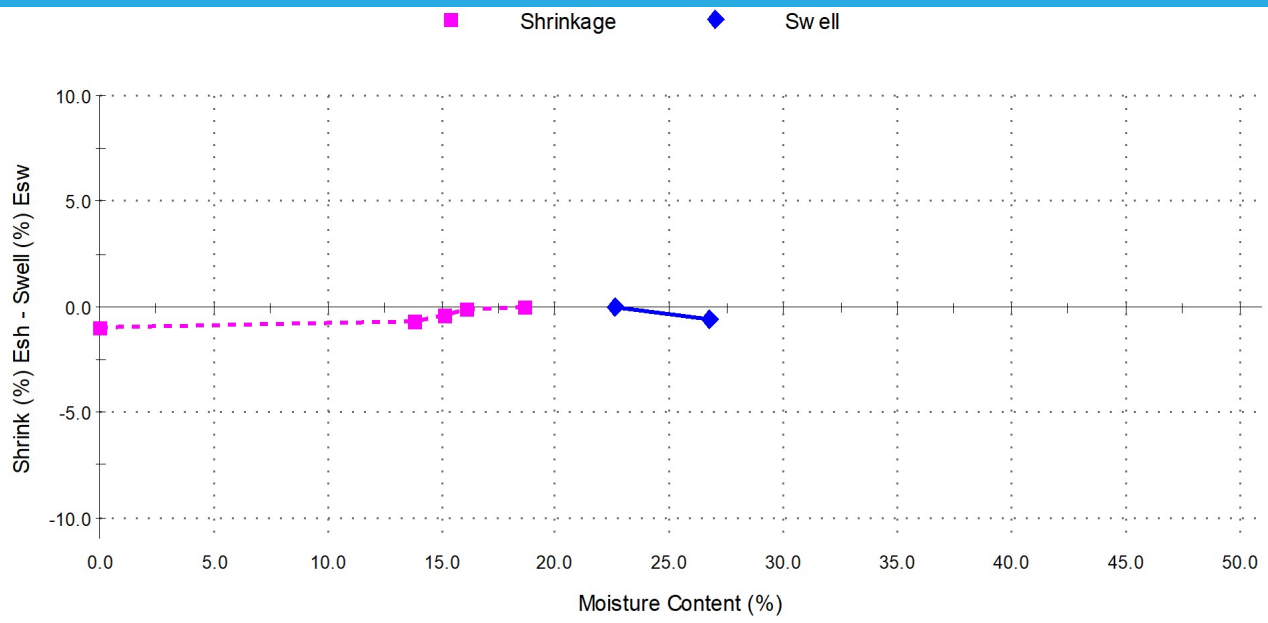
Swell Test AS 1289.7.1.1

Swell on Saturation (%):	-0.6
Moisture Content before (%):	22.6
Moisture Content after (%):	26.7
Est. Unc. Comp. Strength before (kPa):	220
Est. Unc. Comp. Strength after (kPa):	120

Shrink Test AS 1289.7.1.1

Shrink on drying (%):	1.0
Shrinkage Moisture Content (%):	18.6
Est. inert material (%):	4%
Crumbling during shrinkage:	Nil
Cracking during shrinkage:	Major

Shrink Swell



Shrink Swell Index - Iss (%): 0.5

Comments

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2 Murray Dwyer Circuit, Mayfield West, NSW 2304
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F: 02 4960 9775
E: admin@qualtest.com.au
W: www.qualtest.com.au
ABN: 98 153 268 896

Report No: SSI:NEW21W-5267-S04
Issue No: 1

Shrink Swell Index Report

Client: McCloy Project Management Pty Ltd
PO Box 2214
Dangar NSW 2309

Project No.: NEW18P-0170F
Project Name: Brush Creek Subdivision - Precinct 2, Stage 2
Project Location: Bootaring Boulevard, Edgeworth

Accredited for compliance with ISO/IEC 17025-Testing.
The results of the tests, calibrations and/or measurements included in this document are traceable to Australian/national standards.
Results provided relate only to the items tested or sampled.

Approved Signatory: Brent Cullen
(Senior Geotechnician)
NATA Accredited Laboratory Number: 18686
Date of Issue: 21/12/2021

Sample Details

Sample ID: NEW21W-5267-S04
Sampling Method: The results outlined below apply to the sample as received
Material: Gravelly Sandy Clay
Source: On-Site Insitu
Specification: No Specification
Sample Location: BH203 - (1.40 - 1.55m)
Date Tested: 14/12/2021
Date Sampled: 8/12/2021
Date Submitted: 10/12/2021

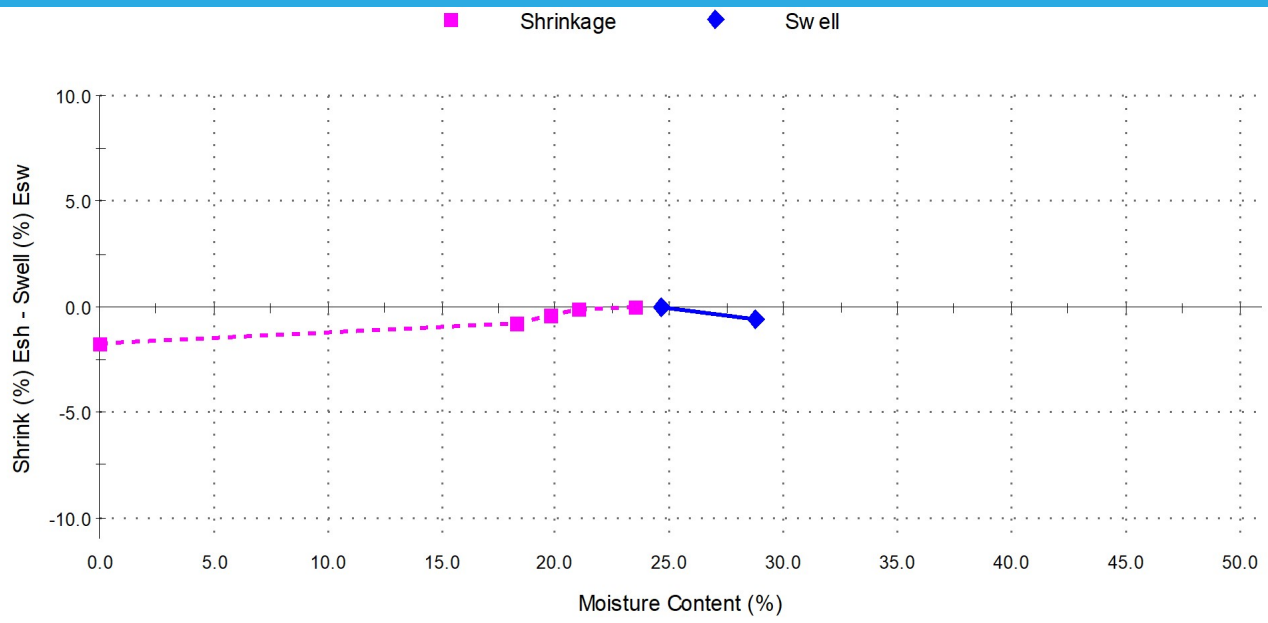
Swell Test AS 1289.7.1.1

Swell on Saturation (%):	-0.6
Moisture Content before (%):	24.6
Moisture Content after (%):	28.8
Est. Unc. Comp. Strength before (kPa):	370
Est. Unc. Comp. Strength after (kPa):	140

Shrink Test AS 1289.7.1.1

Shrink on drying (%):	1.7
Shrinkage Moisture Content (%):	23.5
Est. inert material (%):	5%
Crumbling during shrinkage:	Nil
Cracking during shrinkage:	Major

Shrink Swell



Shrink Swell Index - Iss (%): 0.9

Comments

QUALTEST Laboratory (NSW) Pty Ltd (20708)
2 Murray Dwyer Circuit, Mayfield West, NSW 2304
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F: 02 4960 9775
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W: www.qualtest.com.au
ABN: 98 153 268 896

Report No: SSI:NEW21W-5267-S05
Issue No: 1

Shrink Swell Index Report

Client: McCloy Project Management Pty Ltd
PO Box 2214
Dangar NSW 2309

Project No.: NEW18P-0170F
Project Name: Brush Creek Subdivision - Precinct 2, Stage 2
Project Location: Bootaring Boulevard, Edgeworth

Accredited for compliance with ISO/IEC 17025-Testing.
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Results provided relate only to the items tested or sampled.

Approved Signatory: Brent Cullen
(Senior Geotechnician)
NATA Accredited Laboratory Number: 18686
Date of Issue: 21/12/2021

Sample Details

Sample ID: NEW21W-5267-S05
Sampling Method: The results outlined below apply to the sample as received
Material: Gravelly Sandy Clay
Source: On-Site Insitu
Specification: No Specification
Sample Location: BH204 - (0.80 - 1.00m)
Date Tested: 14/12/2021
Date Sampled: 8/12/2021
Date Submitted: 10/12/2021

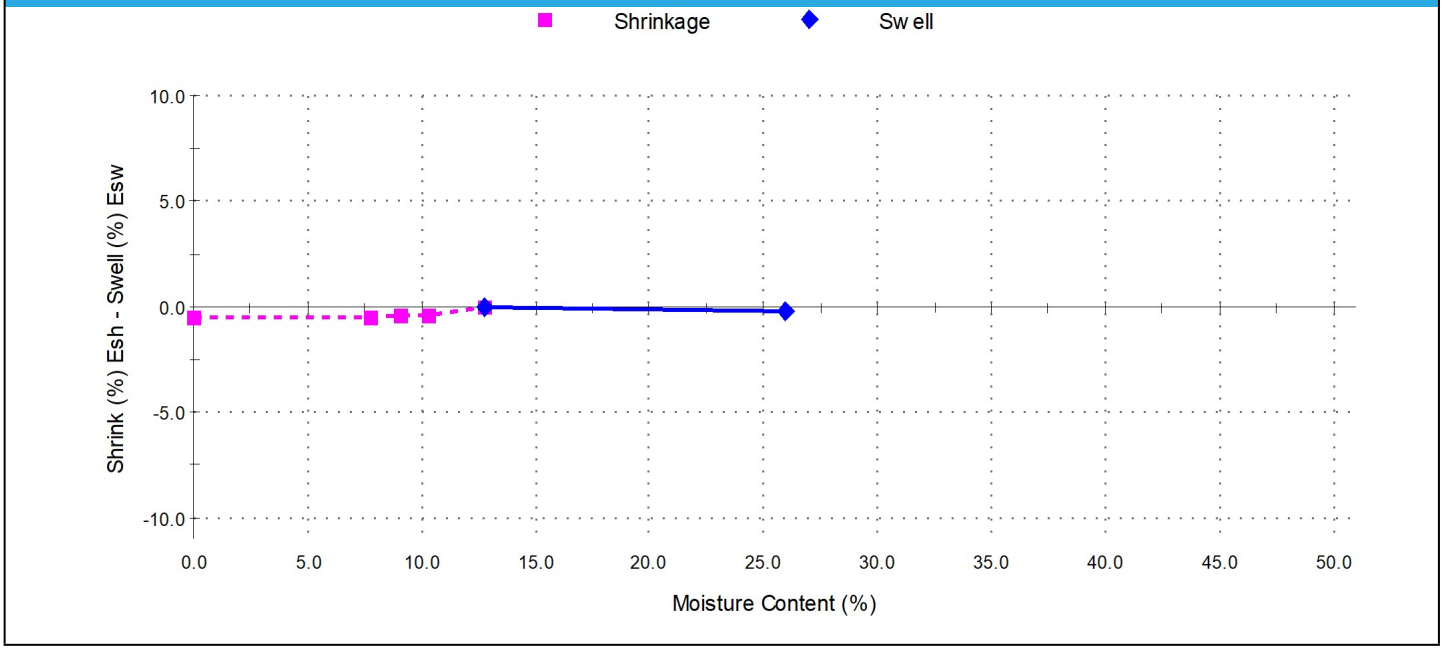
Swell Test AS 1289.7.1.1

Swell on Saturation (%):	-0.2
Moisture Content before (%):	12.8
Moisture Content after (%):	25.9
Est. Unc. Comp. Strength before (kPa):	490
Est. Unc. Comp. Strength after (kPa):	310

Shrink Test AS 1289.7.1.1

Shrink on drying (%):	0.5
Shrinkage Moisture Content (%):	12.8
Est. inert material (%):	3%
Crumbling during shrinkage:	Nil
Cracking during shrinkage:	Minor

Shrink Swell



Shrink Swell Index - Iss (%): 0.3

Comments

QUALTEST Laboratory (NSW) Pty Ltd (20708)
2 Murray Dwyer Circuit, Mayfield West, NSW 2304
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F: 02 4960 9775
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ABN: 98 153 268 896

Report No: SSI:NEW21W-5267-S06
Issue No: 1

Shrink Swell Index Report

Client: McCloy Project Management Pty Ltd
PO Box 2214
Dangar NSW 2309

Project No.: NEW18P-0170F
Project Name: Brush Creek Subdivision - Precinct 2, Stage 2
Project Location: Bootaring Boulevard, Edgeworth

Accredited for compliance with ISO/IEC 17025-Testing.
The results of the tests, calibrations and/or measurements included in this document are traceable to Australian/national standards.
Results provided relate only to the items tested or sampled.

Approved Signatory: Brent Cullen
(Senior Geotechnician)
NATA Accredited Laboratory Number: 18686
Date of Issue: 21/12/2021

Sample Details

Sample ID: NEW21W-5267-S06
Sampling Method: The results outlined below apply to the sample as received
Material: Gravelly Sandy Clay
Source: On-Site Insitu
Specification: No Specification
Sample Location: BH205 - (0.70 - 0.85m)
Date Tested: 14/12/2021
Date Sampled: 8/12/2021
Date Submitted: 10/12/2021

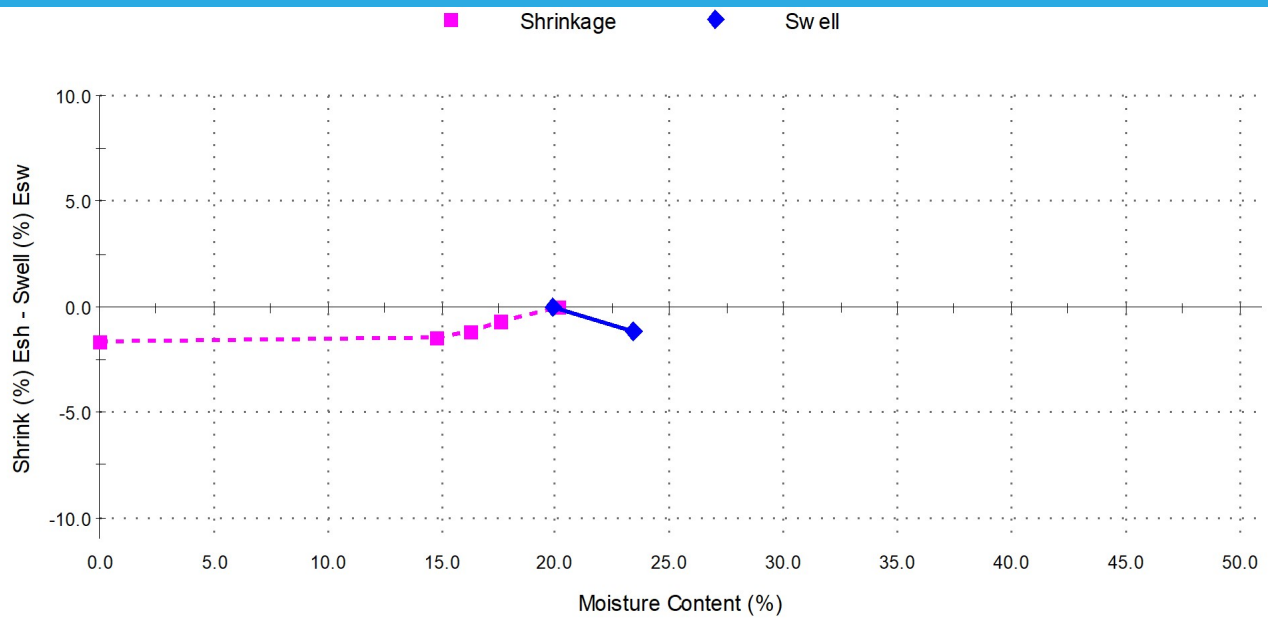
Swell Test AS 1289.7.1.1

Swell on Saturation (%):	-1.2
Moisture Content before (%):	19.8
Moisture Content after (%):	23.4
Est. Unc. Comp. Strength before (kPa):	300
Est. Unc. Comp. Strength after (kPa):	190

Shrink Test AS 1289.7.1.1

Shrink on drying (%):	1.6
Shrinkage Moisture Content (%):	20.2
Est. inert material (%):	6%
Crumbling during shrinkage:	Nil
Cracking during shrinkage:	Moderate

Shrink Swell



Shrink Swell Index - Iss (%): 0.9

Comments

QUALTEST Laboratory (NSW) Pty Ltd (20708)
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F: 02 4960 9775
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W: www.qualtest.com.au
ABN: 98 153 268 896

Report No: MAT:NEW21W-5267-S07
Issue No: 1

Material Test Report

Client: McCloy Project Management Pty Ltd
PO Box 2214
Dangar NSW 2309

Project No.: NEW18P-0170F
Project Name: Brush Creek Subdivision - Precinct 2, Stage 2
Project Location: Bootaring Boulevard, Edgeworth

Accredited for compliance with ISO/IEC 17025-Testing.
The results of the tests, calibrations and/or measurements included in this document are traceable to Australian/national standards.
Results provided relate only to the items tested or sampled.

Approved Signatory: Brent Cullen
(Senior Geotechnician)
NATA Accredited Laboratory Number: 18686
Date of Issue: 11/01/2022

Sample Details

Sample ID: NEW21W-5267-S07
Date Sampled: 08/12/2021
Date Received: 10/12/2021
Source: On-Site Insitu
Material: Gravelly Sandy Clay
Specification: No Specification
The results outlined below apply to the sample as received
Sample Location: BH206 - (0.50 - 0.65m)

Test Results

Description	Method	Result	Limits
Sample History	AS 1289.1.1	Oven-dried	
Preparation	AS 1289.1.1	Dry Sieved	
Linear Shrinkage (%)	AS 1289.3.4.1	8.0	
Mould Length (mm)		250	
Crumbling		No	
Curling		No	
Cracking		No	
Liquid Limit (%)	AS 1289.3.1.1	31	
Method		Four Point	
Plastic Limit (%)	AS 1289.3.2.1	16	
Plasticity Index (%)	AS 1289.3.3.1	15	
Date Tested		10/01/2022	

Comments

N/A

QUALTEST Laboratory (NSW) Pty Ltd (20708)
2 Murray Dwyer Circuit, Mayfield West, NSW 2304
T: 02 4968 4468
F: 02 4960 9775
E: admin@qualtest.com.au
W: www.qualtest.com.au
ABN: 98 153 268 896

Report No: SSI:NEW21W-5267-S08
Issue No: 1

Shrink Swell Index Report

Client: McCloy Project Management Pty Ltd
PO Box 2214
Dangar NSW 2309

Project No.: NEW18P-0170F
Project Name: Brush Creek Subdivision - Precinct 2, Stage 2
Project Location: Bootaring Boulevard, Edgeworth

Accredited for compliance with ISO/IEC 17025-Testing.
The results of the tests, calibrations and/or measurements included in this document are traceable to Australian/national standards.
Results provided relate only to the items tested or sampled.

Approved Signatory: Brent Cullen
(Senior Geotechnician)
NATA Accredited Laboratory Number: 18686
Date of Issue: 21/12/2021

Sample Details

Sample ID: NEW21W-5267-S08
Sampling Method: The results outlined below apply to the sample as received
Material: Gravelly Sandy Clay
Source: On-Site Insitu
Specification: No Specification
Sample Location: BH207 - (0.60 - 0.85m)
Date Tested: 14/12/2021
Date Sampled: 8/12/2021
Date Submitted: 10/12/2021

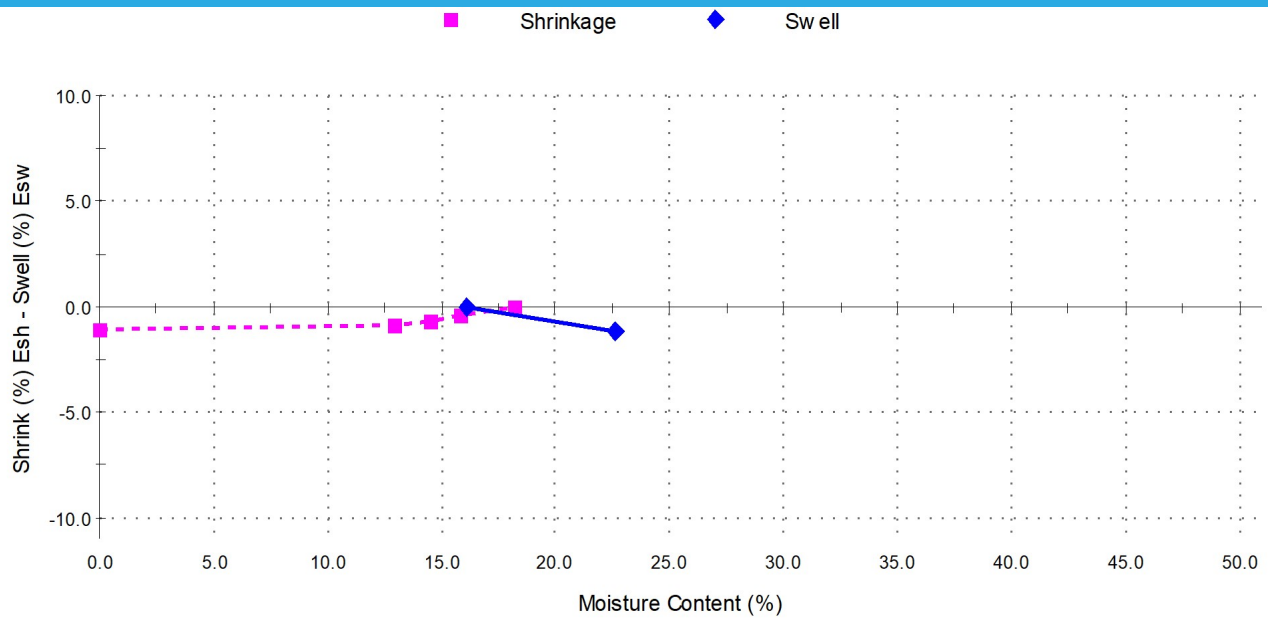
Swell Test AS 1289.7.1.1

Swell on Saturation (%):	-1.2
Moisture Content before (%):	16.0
Moisture Content after (%):	22.6
Est. Unc. Comp. Strength before (kPa):	400
Est. Unc. Comp. Strength after (kPa):	290

Shrink Test AS 1289.7.1.1

Shrink on drying (%):	1.1
Shrinkage Moisture Content (%):	18.2
Est. inert material (%):	8%
Crumbling during shrinkage:	Nil
Cracking during shrinkage:	Minor

Shrink Swell



Shrink Swell Index - Iss (%): 0.6

Comments

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Report No: SSI:NEW21W-5267-S09
Issue No: 1

Shrink Swell Index Report

Client: McCloy Project Management Pty Ltd
PO Box 2214
Dangar NSW 2309

Project No.: NEW18P-0170F
Project Name: Brush Creek Subdivision - Precinct 2, Stage 2
Project Location: Bootaring Boulevard, Edgeworth

Accredited for compliance with ISO/IEC 17025-Testing.
The results of the tests, calibrations and/or measurements included in this document are traceable to Australian/national standards.
Results provided relate only to the items tested or sampled.

Approved Signatory: Brent Cullen
(Senior Geotechnician)
NATA Accredited Laboratory Number: 18686
Date of Issue: 21/12/2021

Sample Details

Sample ID: NEW21W-5267-S09
Sampling Method: The results outlined below apply to the sample as received
Material: Gravelly Sandy Clay
Source: On-Site Insitu
Specification: No Specification
Sample Location: BH208 - (0.60 - 0.80m)
Date Tested: 14/12/2021
Date Sampled: 8/12/2021
Date Submitted: 10/12/2021

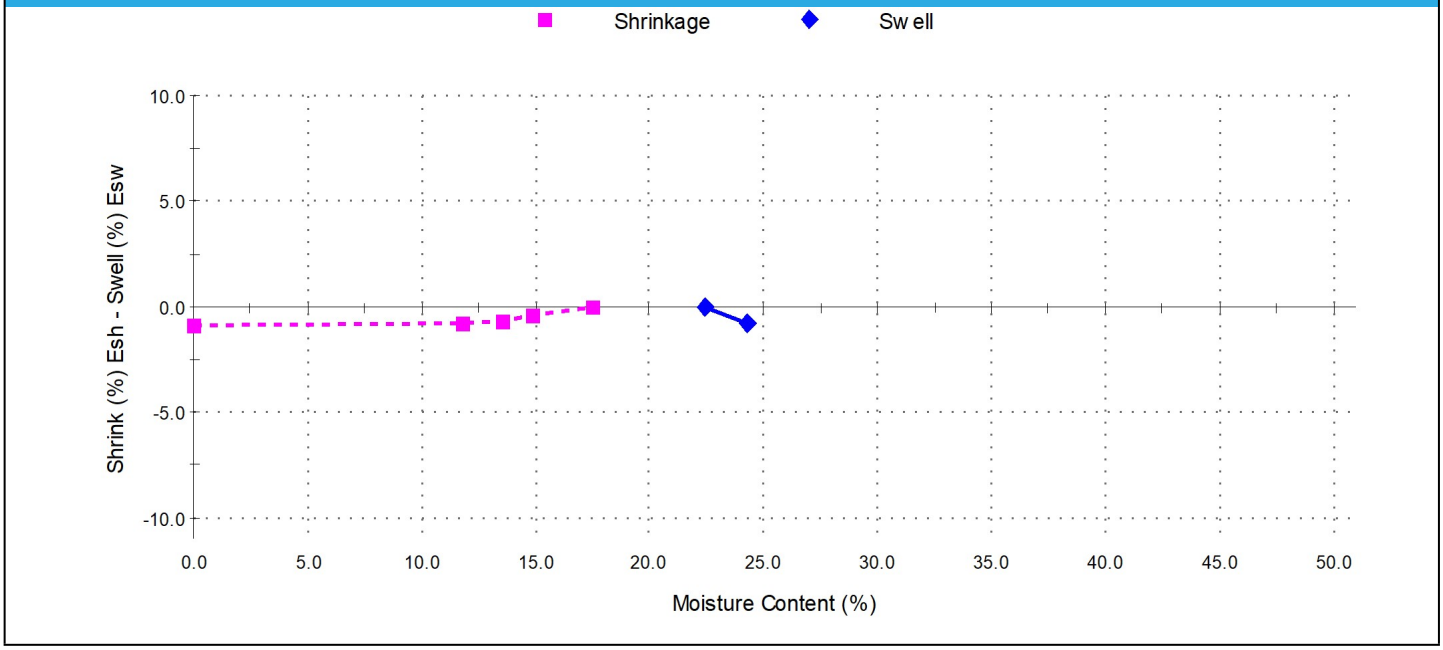
Swell Test AS 1289.7.1.1

Swell on Saturation (%):	-0.8
Moisture Content before (%):	22.4
Moisture Content after (%):	24.3
Est. Unc. Comp. Strength before (kPa):	600
Est. Unc. Comp. Strength after (kPa):	290

Shrink Test AS 1289.7.1.1

Shrink on drying (%):	0.9
Shrinkage Moisture Content (%):	17.5
Est. inert material (%):	8%
Crumbling during shrinkage:	Nil
Cracking during shrinkage:	Minor

Shrink Swell



Shrink Swell Index - Iss (%): 0.5

Comments

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Report No: MAT:NEW21W-5267-S10
Issue No: 1

Material Test Report

Client:	McCloy Project Management Pty Ltd PO Box 2214 Dangar NSW 2309
Project No.:	NEW18P-0170F
Project Name:	Brush Creek Subdivision - Precinct 2, Stage 2
Project Location:	Bootaring Boulevard, Edgeworth

Accredited for compliance with ISO/IEC 17025-Testing.
The results of the tests, calibrations and/or measurements included in this document are traceable to Australian/national standards.
Results provided relate only to the items tested or sampled.

Approved Signatory: Brent Cullen
(Senior Geotechnician)
NATA Accredited Laboratory Number: 18686
Date of Issue: 11/01/2022

Sample Details

Sample ID:	NEW21W-5267-S10
Date Sampled:	08/12/2021
Date Received:	10/12/2021
Source:	On-Site Insitu
Material:	Gravelly Sandy Clay
Specification:	No Specification
	The results outlined below apply to the sample as received
Sample Location:	BH209 - (0.60 - 0.75m)

Test Results

Description	Method	Result	Limits
Sample History	AS 1289.1.1	Oven-dried	
Preparation	AS 1289.1.1	Dry Sieved	
Linear Shrinkage (%)	AS 1289.3.4.1	Not Tested	
Mould Length (mm)			
Crumbling		No	
Curling		No	
Cracking		No	
Liquid Limit (%)	AS 1289.3.1.2	46	
Plastic Limit (%)	AS 1289.3.2.1	17	
Plasticity Index (%)	AS 1289.3.3.1	29	
Date Tested		10/01/2022	

Comments

N/A

APPENDIX C:

CSIRO Sheet BTF 18

Foundation Maintenance and Footing Performance: A Homeowner's Guide

Foundation Maintenance and Footing Performance:
A Homeowner's Guide

CSIRO

BTF 18 replaces Information Sheet 10/91

Buildings can and often do move. This movement can be up, down, lateral or rotational. The fundamental cause of movement in buildings can usually be related to one or more problems in the foundation soil. It is important for the homeowner to identify the soil type in order to ascertain the measures that should be put in place in order to ensure that problems in the foundation soil can be prevented, thus protecting against building movement.

This Building Technology File is designed to identify causes of soil-related building movement, and to suggest methods of prevention of resultant cracking in buildings.

Soil Types

The types of soils usually present under the topsoil in land zoned for residential buildings can be split into two approximate groups – granular and clay. Quite often, foundation soil is a mixture of both types. The general problems associated with soils having granular content are usually caused by erosion. Clay soils are subject to saturation and swell/shrink problems.

Classifications for a given area can generally be obtained by application to the local authority, but these are sometimes unreliable and if there is doubt, a geotechnical report should be commissioned. As most buildings suffering movement problems are founded on clay soils, there is an emphasis on classification of soils according to the amount of swell and shrinkage they experience with variations of water content. The table below is Table 2.1 from AS 2870, the Residential Slab and Footing Code.

Causes of Movement

Settlement due to construction
There are two types of settlement that occur as a result of construction:

- Immediate settlement occurs when a building is first placed on its foundation soil, as a result of compaction of the soil under the weight of the structure. The cohesive quality of clay soil mitigates against this, but granular (particularly sandy) soil is susceptible.
- Consolidation settlement is a feature of clay soil and may take place because of the expulsion of moisture from the soil or because of the soil's lack of resistance to local compressive or shear stresses. This will usually take place during the first few months after construction, but has been known to take many years in exceptional cases.

These problems are the province of the builder and should be taken into consideration as part of the preparation of the site for construction. Building Technology File 19 (BTF 19) deals with these problems.

Erosion
All soils are prone to erosion, but sandy soil is particularly susceptible to being washed away. Even clay with a sand component of say 10% or more can suffer from erosion.

Saturation
This is particularly a problem in clay soils. Saturation creates a bog-like suspension of the soil that causes it to lose virtually all of its bearing capacity. To a lesser degree, sand is affected by saturation because saturated sand may undergo a reduction in volume – particularly imported sand fill for bedding and blinding layers. However, this usually occurs as immediate settlement and should normally be the province of the builder.

Seasonal swelling and shrinkage of soil
All clays react to the presence of water by slowly absorbing it, making the soil increase in volume (see table below). The degree of increase varies considerably between different clays, as does the degree of decrease during the subsequent drying out caused by fair weather periods. Because of the low absorption and expulsion rate, this phenomenon will not usually be noticeable unless there are prolonged rainy or dry periods, usually of weeks or months, depending on the land and soil characteristics.

The swelling of soil creates an upward force on the footings of the building, and shrinkage creates subsidence that takes away the support needed by the footing to retain equilibrium.

Shear failure
This phenomenon occurs when the foundation soil does not have sufficient strength to support the weight of the footing. There are two major post-construction causes:

- Significant load increase.
- Reduction of lateral support of the soil under the footing due to erosion or excavation.
- In clay soil, shear failure can be caused by saturation of the soil adjacent to or under the footing.

GENERAL DEFINITIONS OF SITE CLASSES

Class	Foundation
A	Most sand and rock sites with little or no ground movement from moisture changes
S	Slightly reactive clay sites with only slight ground movement from moisture changes
M	Moderately reactive clay or silt sites, which can experience moderate ground movement from moisture changes
H	Highly reactive clay sites, which can experience high ground movement from moisture changes
E	Extremely reactive sites, which can experience extreme ground movement from moisture changes
A to P	Filled sites
P	Sites which include soft soils, such as soft clay or silt or loose sands; landslip; mine subsidence; collapsing soils; soils subject to erosion; reactive sites subject to abnormal moisture conditions or sites which cannot be classified otherwise

Tree root growth

Trees and shrubs that are allowed to grow in the vicinity of footings can cause foundation soil movement in two ways:

- Roots that grow under footings may increase in cross-sectional size, exerting upward pressure on footings.
- Roots in the vicinity of footings will absorb much of the moisture in the foundation soil, causing shrinkage or subsidence.

Unevenness of Movement

The types of ground movement described above usually occur unevenly throughout the building's foundation soil. Settlement due to construction tends to be uneven because of:

- Differing compaction of foundation soil prior to construction.
- Differing moisture content of foundation soil prior to construction.

Movement due to non-construction causes is usually more uneven still. Erosion can undermine a footing that traverses the flow or can create the conditions for shear failure by eroding soil adjacent to a footing that runs in the same direction as the flow.

Saturation of clay foundation soil may occur where subfloor walls create a dam that makes water pond. It can also occur wherever there is a source of water near footings in clay soil. This leads to a severe reduction in the strength of the soil which may create local shear failure.

Seasonal swelling and shrinkage of clay soil affects the perimeter of the building first, then gradually spreads to the interior. The swelling process will usually begin at the uphill extreme of the building, or on the weather side where the land is flat. Swelling gradually reaches the interior soil as absorption continues. Shrinkage usually begins where the sun's heat is greatest.

Effects of Uneven Soil Movement on Structures

Erosion and saturation

Erosion removes the support from under footings, tending to create subsidence of the part of the structure under which it occurs. Brickwork walls will resist the stress created by this removal of support by bridging the gap or cantilevering until the bricks or the mortar bedding fail. Older masonry has little resistance. Evidence of failure varies according to circumstances and symptoms may include:

- Step cracking in the mortar beds in the body of the wall or above/below openings such as doors or windows.
- Vertical cracking in the bricks (usually but not necessarily in line with the vertical beds or perpends).

Isolated piers affected by erosion or saturation of foundations will eventually lose contact with the bearers they support and may tilt or fall over. The floors that have lost this support will become bouncy, sometimes rattling ornaments etc.

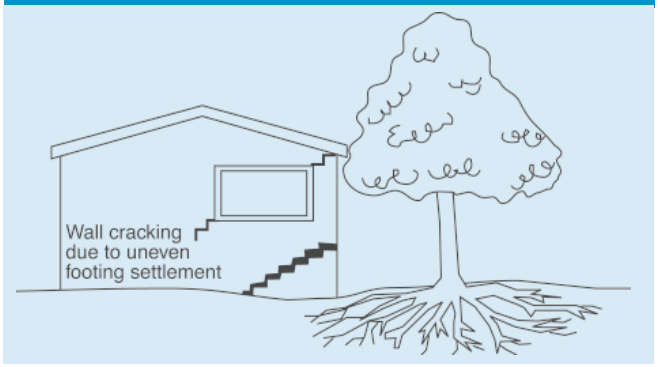
Seasonal swelling/shrinkage in clay

Swelling foundation soil due to rainy periods first lifts the most exposed extremities of the footing system, then the remainder of the perimeter footings while gradually permeating inside the building footprint to lift internal footings. This swelling first tends to create a dish effect, because the external footings are pushed higher than the internal ones.

The first noticeable symptom may be that the floor appears slightly dished. This is often accompanied by some doors binding on the floor or the door head, together with some cracking of cornice mitres. In buildings with timber flooring supported by bearers and joists, the floor can be bouncy. Externally there may be visible dishing of the hip or ridge lines.

As the moisture absorption process completes its journey to the innermost areas of the building, the internal footings will rise. If the spread of moisture is roughly even, it may be that the symptoms will temporarily disappear, but it is more likely that swelling will be uneven, creating a difference rather than a disappearance in symptoms. In buildings with timber flooring supported by bearers and joists, the isolated piers will rise more easily than the strip footings or piers under walls, creating noticeable doming of flooring.

Trees can cause shrinkage and damage



As the weather pattern changes and the soil begins to dry out, the external footings will be first affected, beginning with the locations where the sun's effect is strongest. This has the effect of lowering the external footings. The doming is accentuated and cracking reduces or disappears where it occurred because of dishing, but other cracks open up. The roof lines may become convex.

Doming and dishing are also affected by weather in other ways. In areas where warm, wet summers and cooler dry winters prevail, water migration tends to be toward the interior and doming will be accentuated, whereas where summers are dry and winters are cold and wet, migration tends to be toward the exterior and the underlying propensity is toward dishing.

Movement caused by tree roots

In general, growing roots will exert an upward pressure on footings, whereas soil subject to drying because of tree or shrub roots will tend to remove support from under footings by inducing shrinkage.

Complications caused by the structure itself

Most forces that the soil causes to be exerted on structures are vertical – i.e. either up or down. However, because these forces are seldom spread evenly around the footings, and because the building resists uneven movement because of its rigidity, forces are exerted from one part of the building to another. The net result of all these forces is usually rotational. This resultant force often complicates the diagnosis because the visible symptoms do not simply reflect the original cause. A common symptom is binding of doors on the vertical member of the frame.

Effects on full masonry structures

Brickwork will resist cracking where it can. It will attempt to span areas that lose support because of subsided foundations or raised points. It is therefore usual to see cracking at weak points, such as openings for windows or doors.

In the event of construction settlement, cracking will usually remain unchanged after the process of settlement has ceased.

With local shear or erosion, cracking will usually continue to develop until the original cause has been remedied, or until the subsidence has completely neutralised the affected portion of footing and the structure has stabilised on other footings that remain effective.

In the case of swell/shrink effects, the brickwork will in some cases return to its original position after completion of a cycle, however it is more likely that the rotational effect will not be exactly reversed, and it is also usual that brickwork will settle in its new position and will resist the forces trying to return it to its original position. This means that in a case where swelling takes place after construction and cracking occurs, the cracking is likely to at least partly remain after the shrink segment of the cycle is complete. Thus, each time the cycle is repeated, the likelihood is that the cracking will become wider until the sections of brickwork become virtually independent.

With repeated cycles, once the cracking is established, if there is no other complication, it is normal for the incidence of cracking to stabilise, as the building has the articulation it needs to cope with the problem. This is by no means always the case, however, and monitoring of cracks in walls and floors should always be treated seriously.

Upheaval caused by growth of tree roots under footings is not a simple vertical shear stress. There is a tendency for the root to also exert lateral forces that attempt to separate sections of brickwork after initial cracking has occurred.

The normal structural arrangement is that the inner leaf of brickwork in the external walls and at least some of the internal walls (depending on the roof type) comprise the load-bearing structure on which any upper floors, ceilings and the roof are supported. In these cases, it is internally visible cracking that should be the main focus of attention, however there are a few examples of dwellings whose external leaf of masonry plays some supporting role, so this should be checked if there is any doubt. In any case, externally visible cracking is important as a guide to stresses on the structure generally, and it should also be remembered that the external walls must be capable of supporting themselves.

Effects on framed structures
Timber or steel framed buildings are less likely to exhibit cracking due to swell/shrink than masonry buildings because of their flexibility. Also, the doming/dishing effects tend to be lower because of the lighter weight of walls. The main risks to framed buildings are encountered because of the isolated pier footings used under walls. Where erosion or saturation cause a footing to fall away, this can double the span which a wall must bridge. This additional stress can create cracking in wall linings, particularly where there is a weak point in the structure caused by a door or window opening. It is, however, unlikely that framed structures will be so stressed as to suffer serious damage without first exhibiting some or all of the above symptoms for a considerable period. The same warning period should apply in the case of upheaval. It should be noted, however, that where framed buildings are supported by strip footings there is only one leaf of brickwork and therefore the externally visible walls are the supporting structure for the building. In this case, the subfloor masonry walls can be expected to behave as full brickwork walls.

Effects on brick veneer structures
Because the load-bearing structure of a brick veneer building is the frame that makes up the interior leaf of the external walls plus perhaps the internal walls, depending on the type of roof, the building can be expected to behave as a framed structure, except that the external masonry will behave in a similar way to the external leaf of a full masonry structure.

Water Service and Drainage

Where a water service pipe, a sewer or stormwater drainage pipe is in the vicinity of a building, a water leak can cause erosion, swelling or saturation of susceptible soil. Even a minuscule leak can be enough to saturate a clay foundation. A leaking tap near a building can have the same effect. In addition, trenches containing pipes can become watercourses even though backfilled, particularly where broken rubble is used as fill. Water that runs along these trenches can be responsible for serious erosion, interstrata seepage into subfloor areas and saturation.

Pipe leakage and trench water flows also encourage tree and shrub roots to the source of water, complicating and exacerbating the problem.
Poor roof plumbing can result in large volumes of rainwater being concentrated in a small area of soil:

- Incorrect falls in roof guttering may result in overflows, as may gutters blocked with leaves etc.
- Corroded guttering or downpipes can spill water to ground.
- Downpipes not positively connected to a proper stormwater collection system will direct a concentration of water to soil that is directly adjacent to footings, sometimes causing large-scale problems such as erosion, saturation and migration of water under the building.

Seriousness of Cracking

In general, most cracking found in masonry walls is a cosmetic nuisance only and can be kept in repair or even ignored. The table below is a reproduction of Table C1 of AS 2870.

AS 2870 also publishes figures relating to cracking in concrete floors, however because wall cracking will usually reach the critical point significantly earlier than cracking in slabs, this table is not reproduced here.

Prevention/Cure

Plumbing
Where building movement is caused by water service, roof plumbing, sewer or stormwater failure, the remedy is to repair the problem. It is prudent, however, to consider also rerouting pipes away from the building where possible, and relocating taps to positions where any leakage will not direct water to the building vicinity. Even where gully traps are present, there is sometimes sufficient spill to create erosion or saturation, particularly in modern installations using smaller diameter PVC fixtures. Indeed, some gully traps are not situated directly under the taps that are installed to charge them, with the result that water from the tap may enter the backfilled trench that houses the sewer piping. If the trench has been poorly backfilled, the water will either pond or flow along the bottom of the trench. As these trenches usually run alongside the footings and can be at a similar depth, it is not hard to see how any water that is thus directed into a trench can easily affect the foundation's ability to support footings or even gain entry to the subfloor area.

Ground drainage
In all soils there is the capacity for water to travel on the surface and below it. Surface water flows can be established by inspection during and after heavy or prolonged rain. If necessary, a grated drain system connected to the stormwater collection system is usually an easy solution.

It is, however, sometimes necessary when attempting to prevent water migration that testing be carried out to establish watertable height and subsoil water flows. This subject is referred to in BTF 19 and may properly be regarded as an area for an expert consultant.

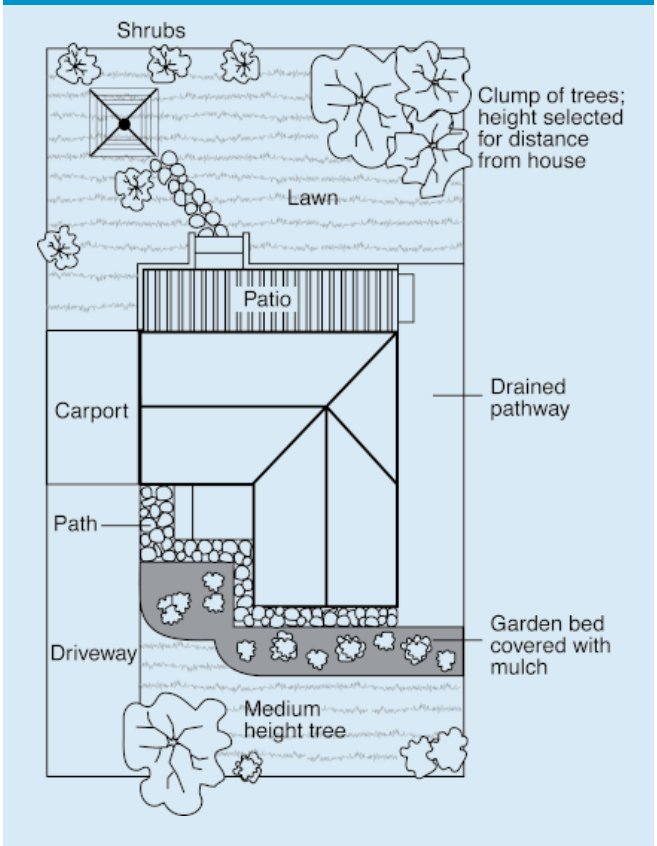
Protection of the building perimeter
It is essential to remember that the soil that affects footings extends well beyond the actual building line. Watering of garden plants, shrubs and trees causes some of the most serious water problems.

For this reason, particularly where problems exist or are likely to occur, it is recommended that an apron of paving be installed around as much of the building perimeter as necessary. This paving

CLASSIFICATION OF DAMAGE WITH REFERENCE TO WALLS

Description of typical damage and required repair	Approximate crack width limit (see Note 3)	Damage category
Hairline cracks	<0.1 mm	0
Fine cracks which do not need repair	<1 mm	1
Cracks noticeable but easily filled. Doors and windows stick slightly	<5 mm	2
Cracks can be repaired and possibly a small amount of wall will need to be replaced. Doors and windows stick. Service pipes can fracture. Weathertightness often impaired	5–15 mm (or a number of cracks 3 mm or more in one group)	3
Extensive repair work involving breaking-out and replacing sections of walls, especially over doors and windows. Window and door frames distort. Walls lean or bulge noticeably, some loss of bearing in beams. Service pipes disrupted	15–25 mm but also depend on number of cracks	4

Gardens for a reactive site



should extend outwards a minimum of 900 mm (more in highly reactive soil) and should have a minimum fall away from the building of 1:60. The finished paving should be no less than 100 mm below brick vent bases.

It is prudent to relocate drainage pipes away from this paving, if possible, to avoid complications from future leakage. If this is not practical, earthenware pipes should be replaced by PVC and backfilling should be of the same soil type as the surrounding soil and compacted to the same density.

Except in areas where freezing of water is an issue, it is wise to remove taps in the building area and relocate them well away from the building – preferably not uphill from it (see BTF 19).

It may be desirable to install a grated drain at the outside edge of the paving on the uphill side of the building. If subsoil drainage is needed this can be installed under the surface drain.

Condensation

In buildings with a subfloor void such as where bearers and joists support flooring, insufficient ventilation creates ideal conditions for condensation, particularly where there is little clearance between the floor and the ground. Condensation adds to the moisture already present in the subfloor and significantly slows the process of drying out. Installation of an adequate subfloor ventilation system, either natural or mechanical, is desirable.

Warning: Although this Building Technology File deals with cracking in buildings, it should be said that subfloor moisture can result in the development of other problems, notably:

- Water that is transmitted into masonry, metal or timber building elements causes damage and/or decay to those elements.
- High subfloor humidity and moisture content create an ideal environment for various pests, including termites and spiders.
- Where high moisture levels are transmitted to the flooring and walls, an increase in the dust mite count can ensue within the living areas. Dust mites, as well as dampness in general, can be a health hazard to inhabitants, particularly those who are abnormally susceptible to respiratory ailments.

The garden

The ideal vegetation layout is to have lawn or plants that require only light watering immediately adjacent to the drainage or paving edge, then more demanding plants, shrubs and trees spread out in that order.

Overwatering due to misuse of automatic watering systems is a common cause of saturation and water migration under footings. If it is necessary to use these systems, it is important to remove garden beds to a completely safe distance from buildings.

Existing trees

Where a tree is causing a problem of soil drying or there is the existence or threat of upheaval of footings, if the offending roots are subsidiary and their removal will not significantly damage the tree, they should be severed and a concrete or metal barrier placed vertically in the soil to prevent future root growth in the direction of the building. If it is not possible to remove the relevant roots without damage to the tree, an application to remove the tree should be made to the local authority. A prudent plan is to transplant likely offenders before they become a problem.

Information on trees, plants and shrubs

State departments overseeing agriculture can give information regarding root patterns, volume of water needed and safe distance from buildings of most species. Botanic gardens are also sources of information. For information on plant roots and drains, see Building Technology File 17.

Excavation

Excavation around footings must be properly engineered. Soil supporting footings can only be safely excavated at an angle that allows the soil under the footing to remain stable. This angle is called the angle of repose (or friction) and varies significantly between soil types and conditions. Removal of soil within the angle of repose will cause subsidence.

Remediation

Where erosion has occurred that has washed away soil adjacent to footings, soil of the same classification should be introduced and compacted to the same density. Where footings have been undermined, augmentation or other specialist work may be required. Remediation of footings and foundations is generally the realm of a specialist consultant.

Where isolated footings rise and fall because of swell/shrink effect, the homeowner may be tempted to alleviate floor bounce by filling the gap that has appeared between the bearer and the pier with blocking. The danger here is that when the next swell segment of the cycle occurs, the extra blocking will push the floor up into an accentuated dome and may also cause local shear failure in the soil. If it is necessary to use blocking, it should be by a pair of fine wedges and monitoring should be carried out fortnightly.

This BTF was prepared by John Lewer FAIB, MIAMA, Partner, Construction Diagnosis.

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